

V.3.3-FLDWAV GENERALIZED FLOOD WAVE ROUTING OPERATION

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Identifier: FLDWAV

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Application: All programs

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Description: Operation FLDWAV is a generalized flood routing model that can be used for real-time flood forecasting of dam-break floods and/or natural floods, dam-breach flood analysis for sunny-day piping or overtopping associated with the Probable Maximum Flood, floodplain inundation mapping for contingency dam-break flood planning and design of waterway improvements.

The model computes the outflow hydrograph from a dam due to spillway, overtopping and/or dam breach outflows. The resulting flood wave is then routed through the downstream channel/valley using a four-point

implicit finite-difference numerical solution of the complete Saint-Venant equations of one-dimensional unsteady flow along with appropriate internal boundary equations representing downstream dams, bridges, weirs, waterfalls and other man-made/natural flow controls. The flow may be 'mixed' (subcritical and/or supercritical) throughout the downstream routing reach.

The following features are in Operation FLDWAV:

- o the flood may occur in a system of interconnected rivers such as the main-stem river and its tributaries
- o levee-overtopping/crevasse flows into and through levee protected flood plains
- o automatic calibration of Manning roughness coefficients for historical floods
- o use of multiple routing techniques throughout the river system
- o create output files to be used by program FLDGRF to display model output

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Allowable Time Intervals: 1, 2, 3, 4, 6, 8, 12 and 24 hours

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Time Series Used:

<u>General Type</u>	<u>Dimn</u>	<u>Units</u>	<u>Use</u>	<u>Required</u>	<u>Form of Output T.S.</u>	<u>Data Time Interval</u>	<u>Missing Values Allowed</u>
Observed Stage	L	M	I	no	n/a	any <u>2</u> /	yes
Observed Discharge	L3/T	CMS	I	no	n/a	any <u>2</u> /	yes
Lateral Inflow	L3/T	CMS	I	no	n/a	any <u>2</u> /	no
Target Pool Elevation	L	M	I	no	n/a	any <u>2</u> /	yes
Gate Control Switches	DLES	INT	I	no	n/a	any <u>2</u> /	yes
Upstream Stage	L	M	I	yes <u>1</u> /	n/a	any <u>2</u> /	no
Upstream Discharge	L3/T	CMS	I	yes <u>1</u> /	n/a	any <u>2</u> /	no
Downstream Stage	L	M	I	no	n/a	any <u>2</u> /	no

<u>General Type</u>	<u>Dimn</u>	<u>Units</u>	<u>Use</u>	<u>Required</u>	<u>Form of Output T.S.</u>	<u>Data Time Interval</u>	<u>Missing Values Allowed</u>
Downstream Discharge	L3/T	CMS	I	no	n/a	any <u>2/</u>	no
Output Stage	L	M	O	no	n/a	any <u>3/</u>	no
Output Discharge	L3/T	CMS	O	no	n/a	any <u>3/</u>	no
Output Velocity	L/T	M/S	O	no	n/a	any <u>3/</u>	no
Observed Tide	L	M	I	no	n/a	any	no
NOS Tide	L	M	I	no	n/a	any	no
Adjusted Tide	L	M	O	no	n/a	any	no
Adjusted Stage	L	M	O	no	n/a	any	no
Adjusted Discharge	L3/T	M	O	no	n/a	any	no

1/ These time series must be the same type as the upstream boundaries.

2/ All of these time series must have the same data time interval.

3/ These time series data time interval must be an even increment of the input time series data time interval.

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Input Summary: The syntax rules for the input for this Operation are as follows:

- o values are input in free format (values separated by blanks or a comma)
- o no data may be entered beyond column 72
- o enter a zero if no value is to be specified
- o decimal points are needed only if the value contains a decimal point
- o a blank line or a comment line must be entered before each data group except data groups 0-1 and 0-2; the comments are not saved and therefore are not included when the Segment definition is output)

'*' indicates the Data Group (DG) is required for any simulation.

'**' indicates the Data Group (DG) is required for any dam break simulation.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
0-1*	MSG	Description of the data set. A maximum of 20 lines is allowed, the last line must be EOM. Each line may have a maximum of 72 characters.
0-2*	DESC	Type of output display. For echo print of the input parameters, enter 'NODESC'. For a description of the model parameters enter 'DESC'.
1*	EPSY	Depth tolerance in Newton-Raphson Iteration scheme (0.001-1.0 FT). A good value is 0.01 FT.
	THETA	Acceleration factor in solving tributary junction problem (0.5-1.0). Varies with each problem. A good first choice is 0.8.
	F1	θ weighting factor (0.5-1.0) in finite difference technique. A good value is 0.6.
	XFACT	Factor to convert units describing the location of the computational points along the routing reach to feet; e.g., if units are in (units of MI) , XFACT=5280. When using metric units, this factor converts the units to meters: e.g., if units are in(km),XFACT=1000.
	DTHYD	Time interval (units of HR) of all input hydrographs. If time interval is not constant then set DTHYD=0. If running in NWSRFS (not in stand-alone mode) then set DTHYD>0.
	DTOUT	Time interval (units of HR) of all output hydrographs. If running in stand-alone mode (not a part of NWSRFS) then set DTOUT=0.
	METRIC	Parameter indicating if input/output is in English (METRIC=0) or Metric (METRIC=1) units. All computations within FLDWAV are done in English units; only the input/output may be displayed in metric units. See Table 1 [<u>Bookmark</u>] for units conversion information.
2*	JN	Total number of rivers in the system being routed simultaneously.
	NU	Number of values associated with observed

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		hydrographs.
	ITMAX	Maximum number of iterations allowed in the Newton-Raphson Iteration scheme for solving the system of nonlinear equations. If ITMAX=1, the nonlinear formation degenerates into a linear formation and no iterations are required in the Newton-Raphson iteration procedure. A good value is 10.
	KWARM	Number of time steps used for warm-up procedure. If KWARM=0, no warm-up is done. If KWARM> 0, the model assumes steady-state initial conditions and will solve the routing equations KWARM times without incrementing the time variable. A good value is 2. If running in NWSRFS (not stand-alone mode) or if initial conditions are not steady-state then set KWARM=0.
	KFLP	Parameter indicating the use of the floodplain (conveyance) option. If KFLP=0, no floodplain defined (composite channel used); if KFLP=1, floodplain used with conveyance (K) generated; if KFLP>2, floodplain used with K values read in and KFLP is the number of points in the conveyance table.
	NET	Parameter indicating the use of the channel network option. If NET=0, the network option is not used and a dendritic tree-type system is modeled using the relaxation algorithm. The network option is currently unavailable; set NET=0.
	ICOND	Parameter indicating the type of initial conditions. If initial conditions were not modified and will not be read in then set ICOND=0. If initial conditions are read-in then set ICOND=1 and initial conditions at interpolated cross sections will be interpolated between the read-in values. If running in stand-alone mode (not a part of NWSRFS) then set ICOND=0.
	NMAP	Number of flood inundation mapping scenarios. If no flood mapping in the system then set NMAP=0.
	IFUT(2)	Future parameters; enter three zero values for future enhancements.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
3*	NYQD	Number of sets of stage-discharge values in empirical rating curve at downstream boundary.
	KCG	Number of data points in spillway gate control curve of gate opening (GHT) versus time (TGHT) (DG-38,DG-39). If no movable gates in the system then set KCG=0.
	NCG	Maximum number of movable gates on any single dam in the system (ICG=2, DG-29). If no movable gates in the system then set NCG to 0.
	KPRES	Parameter indicating method of computing hydraulic radius R . If KPRES=0, then $R=A/B$ where A is cross-sectional flow area and B is channel top width; if KPRES=1, then $R=A/P$ where P is wetted perimeter.
4*	NCS	Number of values in table of top width (BS) versus elevation (HS). This value applies to all cross sections in the river system.
	KPL	Parameter indicating what information will be plotted. If KPL=0, nothing is plotted; if KPL=1, water surface elevation (units of FT above Mean Sea Level) hydrographs are plotted; if KPL=2, discharge hydrographs are plotted; if KPL=3, both are plotted. This parameter has nothing to do with the FLDGRF utility. If running in NWSRFS (not in stand-alone mode) and $KPL < 0$, stages (units of FT) will be plotted instead of elevations.
	JNK	Parameter indicating if hydraulic information will be printed. If JNK=0, nothing will be printed; if $JNK > 0$, hydraulic information will be printed; if $JNK < 0$, hydraulic information will be printed for specified reaches. See Table 2 [Bookmark] for description of intermediate analysis output. A good value is JNK=4 or 5.
	KREVRS	Parameter indicating use of the low flow filter. If KREVRS=0, the low flow filter is activated preventing the water surface elevations (WSELs) and discharges from going below the initial condition values; if KREVRS=1, the low flow filter is off and reverse flow is allowed.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	NFGRF	Parameter indicating if data needed for the FLDGRF utility will be generated. If NFGRF=0, the data will be generated; if NFGRF=1, the data will not be generated.
5*	IOBS	Parameter indicating if observed data are available at gaging stations. If IOBS=0, no data available; if IOBS=1, data is available; if IOBS=2, observed data are available and the forecast stages will be adjusted using Manning n ranges; if IOBS=3, observed data are available and the forecast stages will be adjusted using specified balances; if IOBS=-1, a mathematical function is used to describe the inflow hydrograph. If running in stand-alone mode (not a part of NWSRFS), IOBS must be less than 2.
	KTERM	Parameter indicating if the terms in equation of motion will be printed as special information. If KTERM=0, they will not be printed; if KTERM=1, they will be printed. Normally use KTERM=0.
	NP	Parameter indicating if Automatic Calibration option is used. If NP=0, calibration is not used; if NP=-1, automatic calibration of the roughness coefficient (n) is done; if NP=-4, automatic calibration of n using average cross sections is done.
	NPST	Parameter indicating the first value in the computed stage hydrograph which will be used in the statistics needed in the automatic calibration option to determine the Manning n. If NPST=0, the first value of observed stage hydrograph will be used. If NP=0 then set NPST=0.
	NPEND	Parameter indicating the last value in the computed stage hydrograph which will be used in the statistics needed in the automatic calibration option to determine the Manning n. If NPEND=0, the last value of observed stage hydrograph will be used. If NP=0 then set NPEND=0.
Skip DG-6 if JNK is greater than or equal to 0.		
6	TDBG1	Time at which additional intermediate analysis information begins.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	TDBG2	Time at which additional intermediate analysis information ends.
	JNKDBG	Intermediate analysis output indicator (JNK, DG-4). See Table 2 for available intermediate analysis output types.
	JDBG1	First river at which additional intermediate analysis information will be applied.
	JDBG2	Last river at which additional intermediate analysis information will be applied.
	LDBG1	First reach at which intermediate analysis information will be applied during calibration. If NP=0 (DG-5), LDBG1 is the first cross section where intermediate analysis will be applied during simulation.
	LDBG2	Last reach at which intermediate analysis information will be applied during calibration. After this reach has been calibrated, the model will stop. If NP=0 (DG-5), LDBG2 is the last cross section where intermediate analysis will be applied during simulation.
	MCMDBG	First iteration during calibration at which intermediate analysis information will be printed. If NP=0 (DG-5) then set MCMDBG=0.
7*	THE	Time (units of HR) at which routing computations will terminate. If running in NWSRFS (not stand-alone mode) then set THE=0.
	DTHII	Initial computational time step. If DTHII>0, a constant time step is used; if DTHII=0, a variable time step is used based on the inflow hydrographs and dam failure times. If DTHII<0, an array of time steps (NDT values) will be read in where NDT is the absolute value of DTHII.
	DTHPLT	Time step (units of HR) at which computed/observed hydrograph data are stored for plotting or printing. If DTHPLT=0 then set DTHPLT=DTHII. If KPL=0 (DG-4) then set DTHPLT=0.
	FRDFR	Window for critical Froude number in mixed-flow algorithm. Froude numbers in the range of (1+/- FRDFR) will be treated as though the

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		Froude number is equal to 1. The default value is 0.05.
	DTEXP	Computational time step (units of HR) for explicit routing. If DTEXP>0, then a constant time step is used. If DTEXP<0, then a variable time step is used based on the Courant number (C_n) where C_n is the absolute value of DTEXP. If explicit routing is not used then set DTEXP=0.
	MDT	Divisor for determining the time step ($\Delta t = t_p / \text{MDT}$). A good value is 20 for subcritical flow or 40 for supercritical flow. If a constant time step is read-in (DTHII not equal to 0) then set MDT=0.
Skip DG-8 and DG-9 if time step array is not used (DTHII greater than or equal 0).		
8	DTHIN(K)	Computational time step to be used until time TDTIN(K). K index goes from 1 to NDT (DG-7).
9	TDTIN(K)	Time at which DTHIN(K) is no longer used. K index goes from 1 to NDT (DG-7).
10*	NLEV	Total number of cross-section reaches in the system that have levees.
	DHLV	The difference between the maximum and minimum crest elevations along the reach (this is sometimes useful to prevent numerical problems with suddenly large outflows when the levee is first overtopped. If NLEV=0 then set DHLV to zero.
	DTHLV	Computational time step to be used during levee overtopping/failure. If NLEV=0 then set DTHLV=0.
Skip DG-11 if no levees in the system (NLEV=0).		
11	NJFM(K)	Sequence number of river from which levee overtopping/failure flow is passed from reach K.
	NIFM(K)	Sequence number of reach along the river from which levee flow passing into reach NITO(K).
	NJTO(K)	Sequence number of river or pond receiving flow from levee overtopping/failure in reach K.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	NITO(K)	Sequence number of the reach along the river receiving flow from reach NIFM(K). If the receiving channel is a pond (i.e., level pool routing done) then set NITO(K)=0.

Repeat DG-11 for each levee reach (K=1,NLEV).

Skip DG-11a if no flood mapping in the system (NMAP=0).

11a	MPRV(L)	Sequence number of main river where water surface profile will be mapped.
	MPLOC(1,L)	Sequence number of first cross section in the reach to be mapped.
	MPLOC(2,L)	Sequence number of last cross section in the reach to be mapped.
	DTMAP	Time step for flood map animation. This value is only read from the first mapping scenario (L=1). Therefore, it need not be entered for subsequent scenarios.
	SYSPATH(L)	River system name.
	TWNPTH(L)	Scenario name.

WARNING: The river system name and scenario name values are used with the fldview_dir token to define the directory in which the FLDVIEW data files are written. If the combination is not unique then the files could be overwritten. For example a Carryover Group is run which contains two Forecast Groups that have the same river system and scenario names but have different cross sections associated with them then the information for both scenarios will be written to the same directory but the data files will contain the information for the last scenario.

Repeat DG-11a for each mapping scenario (L=1,NMAP).

12*	NBT(J)	Total number of actual cross sections on river J.
	NPT(1,J)	Beginning cross-section number (after interpolation) on river J for which intermediate analysis information will be printed. This parameter is required when JNK is greater than or equal to 9 .
	NPT(2,J)	Final cross-section number (after interpolation) on river J for which intermediate analysis information will be

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		printed. This parameter is required when JNK is greater than or equal to 9.
	MRV(J)	Number of river into which river J flows. Omit this field for main river (J=1). Note that tributary (J-1) is river J.
	NJUN(J)	Sequence number of cross section immediately upstream of tributary (J-1) confluence (this section coincides with the upstream extremity of the small sub-reach which is equivalent in length to the tributary width). Omit this field for main river (J=1).
	ATF(J)	Azimuth angle (degrees) that tributary J makes with the main river at the confluence. Omit this field for main river (J=1).
	EPQJ(J)	Discharge tolerance in Newton-Raphson Iteration scheme in main river (J=1) or in Tributary Iteration Scheme (J>1).
	COFW(J)	Coefficient of wind stress (1.1E-06 to 3.0E-06) on river J.
	VWIND(J)	Wind velocity (units of FT/S) on river J; (+) if directed upstream; (-) if directed downstream.
	WINAGL(J)	Acute angle (degrees) that wind makes with the channel axis of river J.
Repeat DG-12 for each river (J=1,JN).		
13*	KU(J)	Parameter indicating the type of upstream boundary condition being specified for the main river and tributaries; if KU(J)=1, a stage hydrograph is the upstream boundary condition; if KU(J)=2, a discharge hydrograph is the upstream boundary condition.
	KD(J)	Parameter indicating the type of downstream boundary condition being specified for the main river (KD(1)) and the tributaries (KD(J) where J goes from 2 to JN); if KD(1)=0, an observed tide hydrograph is specified which will be blended with a simulated tide hydrograph; if KD(1) or KD(J)=1, a stage hydrograph is the downstream boundary condition; if KD(1)=2, a discharge hydrograph is the downstream boundary condition; if KD(1)=3, a single-valued rating curve of

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		discharge as a function of stage is the boundary condition; if KD(1)=4, a looped rating curve is generated based on Manning's equation where the friction slope is computed based on the momentum equation; if KD(1)=5, normal flow computed from Manning's equation is the downstream boundary condition; if KD(1)=7, a looped rating curve is generated where the friction slope is computed based on conveyance; if KD(1)=1 and NYQD>0, a single-valued rating curve in which Q is a function of the computed water surface minus the read-in value of STN is the boundary condition. If running in stand-alone mode (not a part of NWSRFS), KD(1) must be greater than zero. In the case of tributaries, a stage hydrograph is generated at the downstream boundary and KD(J) is always equal to zero.
	NQL(J)	Total number of lateral flows on river J.
	NGAGE(J)	Total number of observed time series along river J (routing reach) which will be compared with computed time series; also, denotes total number of stations for which computed values will be plotted independently of FLDGRF.
	NRCM1(J)	Total number of Manning n reaches on river J.
	NQCM(J)	Total number of values in the Manning n table. Also, denotes whether Manning n is a function of water surface elevation (NQCM(J)>0) or discharge (NQCM(J)<0). If NQCM(J)=0, Manning n is a function of water surface elevation and the number of table values is equal to NCS.
	NSTR(J)	Total number of computed time series (stage, discharge, or velocity) to be stored on each river. (Number of output time series on each river). If running in stand-alone mode (not a part of NWSRFS) then set NSTR(J)=0.
	IFUT(3)	Future parameters; enter three zero values for future enhancements.
	Repeat DG-13 for each river (J=1,JN).	
14*	MIXF(J)	Parameter indicating the flow regime in river J. If MIXF(J)=0, river J has subcritical flow; if MIXF(J)=1, river J has supercritical

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		flow; if MIXF(J)>1, there is a mixture of subcritical and supercritical flow throughout river J at varying times; if MIXF(J)=2, the hydraulic jump can move upstream or downstream; if MIXF(J)=3, the hydraulic jump moves only if the Froude number exceeds 2; if MIXF(J)=4, the hydraulic jump is stationary; if MIXF(J)=5, a modified implicit technique (LPI) is used to solve mixed flows.
	MUD(J)	Parameter indicating the use of the mud/debris flow option on river J. If MUD(J)=0, dynamic routing of non-mudflow (water) will be done; if MUD(J)=1, dynamic routing of mudflow will be done.
	KFTR(J)	Parameter indicating the use of Kalman filter option on river J. If KFTR(J)=0, Kalman filter option is not used; if KFTR(J)=1, Kalman filter option will be used. Kalman filter can be turned on to update the forecast if river J has stage observations for more than 2 gaging stations.
	KLOS(J)	Parameter indicating the computation of volume losses in river J. If KLOS(J)=0, the losses will not be computed; if KLOS(J)=1, the losses will be computed.
	IFUT(6)	Future parameters; enter six zero values for future enhancements.

Repeat DG-14 for each river (J=1,JN).

Skip DG-15 if LPI technique is not used in system (all MIXF(J)'s are not equal to 5).

15	KLPI(K)	Power (m) used in the LPI technique. Values range from 1 to 10 where m=10 approaches the fully dynamic technique and m=1 approaches the diffusion technique. K index goes from 1 to the number of rivers using the LPI technique. A good value is 5.
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Skip DG-16 if MUDFLOW option is not used in system (all MUD(J)'s=0).

16	UW1(J)	Unit weight (units of lb/FT ³) of mud/debris fluid on river J.
	VIS1(J)	Dynamic viscosity (units of lb-sec/FT ²) of mud/debris fluid in river J.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	SHR1(J)	Initial yield stress of shear strength (units of lb/FT ²) of mud/debris fluid on river J.
	POWR1(J)	Exponent in power function representing the stress-rate of strain relation of fluid in river J; if Bingham plastic is assumed for fluid then set POWR1(J)=1.0.
	IWF1(J)	Parameter indicating dry bed routing on river J. If IWF1(J)=0, the base flow at t=0 will be used all along the routing reach; if IWF1(J)>0, wave front tracking will be used where the wave front velocity (V_w) is a function of the channel velocity (V); if IWF1(J)=1, $V_w = V_{N-4}$; if IWF1(J)=2, $V_w = (K_w)(V_{N-4})$; if IWF1(J)=3, $V_w = V_{C_{MAX}}$, where C_{MAX} is the maximum velocity in the channel reach, N is the current location of the wave front and K_w is the kinematic wave factor.

Repeat DG-16 for each river with mudflow (MUD(J)>0, J=1,JN).

Skip DG-17 if volume flow losses are not computed in system (all KLOS(J)'s=0).

17	XL0S(1,J)	Beginning location (units of MI) of the reaches where flow loss will occur on river J.
	XL0S(2,J)	Ending location (units of MI) of the reaches where flow loss will occur on river J.
	QL0S(J)	Percentage of the loss in terms of total active flow amount; (-) for loss and (+) for gain.
	AL0S(J)	Loss distribution coefficient for river J (0.3-3.0). For a linear loss distribution then set AL0S(J)=1.

Repeat DG-17 for each river with volume flow losses (KLOS(J)>0, J=1,JN).

18*	XT(I,J)	Location of station or cross section where computations are made (units can be anything since XFACT converts these units to FT); I index goes from 1 to NBT(J).
19*	DXM(I,J)	Minimum computational distance step between cross sections. If DXM(I,J) is less than the distance between two adjacent cross sections read in, then intermediate cross sections are

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		created within the program via a linear interpolation procedure. I index goes from 1 to NBT(J)-1.
20*	KRCHT(I,J)	Parameter indicating routing method or internal boundary condition in each reach. See Table 3 [Bookmark] for a description of each type. I index goes from 1 to (NBT(J)-1).

Repeat DG-18 through DG-20 for each river (J=1,JN).

Skip DG-21 through DG-25 if NLEV=0.

21	HWLV(L)	Elevation (units of FT above Mean Sea Level) of top of levee, ridge line, etc. where weir-flow occurs. This elevation is located on the upstream end of the levee reach. If HWLV(L)<0, discharge flows through a pipe and the absolute value of HWLV(L) is the invert elevation of pipe.
	WCLV(L)	Weir-flow discharge coefficient for Δx reach where weir flow (inflow or outflow) may occur. Coefficient ranges from 2.6 to 3.2; if there is a pipe connection (HWLV(L)<0), the weir coefficient=the absolute value of (8.02 times the discharge loss coefficient times the maximum area of the pipe).
	TFLV(L)	Time (units of HR) from start of levee failure (crevasse) until the opening or breach is its maximum size. Set TFLV(L)=0 if the levee does not fail.
	BLVMX(L)	Final width (units of FT) of levee crevasse which is assumed to have a rectangular shape (200-5000 FT). Set BLVMX(L)=0 if the levee does not fail.
	HFLV(L)	Elevation (units of FT above Mean Sea Level) of water surface when levee starts to fail. Set HFLV(L)=0 if the levee does not fail.
	HLVMN(L)	Final elevation (units of FT above Mean Sea Level) of bottom of levee crevasse. Set HLVMN(L)=0 if the levee does not fail.
	SLV(L)	Slope of levee L (units of FT/FT). This parameter is used to interpolate levee reaches. Interpolation is done from the upstream end of the reach.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
Skip DG-22 if levee has no drainage pipe (HWLV(L) is greater than or equal to 0).		
22	HPLV(L)	Centerline elevation (units of FT above Mean Sea Level) of flood drainage pipe (with flood gate).
	DPLV(L)	Diameter (units of FT) of flood drainage pipe.
Repeat DG-21 and DG-22 for each levee reach (L=1,NLEV).		
Skip DG-23 through DG-25 if no ponds exist in the system (NITO(L)>0, L=1,NLEV).		
23	HPOND(L)	Initial water surface elevation (units of FT above Mean Sea Level) of storage pond L in levee option.
24	SAPOND(K,L)	Surface area (acres) of storage pond L corresponding to elevation HSAP in the area-elevation curve. These values should be entered from the top of the pond (maximum elevation) to the bottom. K index goes from 1 to 8. If less than 8 values are needed to describe the pond then set the remaining values to zero.
25	HSAP(K,L)	Elevation (units of FT above Mean Sea Level) corresponding to SAPOND in the area elevation curve. These values should be entered from the top of the pond (maximum elevation) to the bottom. K index goes from 1 to 8. If less than 8 values are needed to describe the pond then set the remaining values to zero.
Repeat DG-23 through DG-25 for each pond (L=1 to number of ponds).		
Skip DG-26 through DG-47 if no internal boundaries in the system (all KRCHT<10).		
Skip DG-26 through DG-43 if internal boundary K is not a dam (KRCHT(K,J)<10 or KRCHT(K,J)>30).		
Skip DG-26 and DG-27 if internal boundary K is not a reservoir (KRCHT(K,J) is not equal to 4 or [KRCHT(1,J)<10 or KRCHT(1,J)>30]).		
26	SAR(L,K,J)	Surface area (acres) of reservoir behind dam at elevation HSAR(L,K,J). Values should be read in from the top of the reservoir to the bottom of the reservoir. L index goes from 1 to 8; if less than 8 values are needed to

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		describe the reservoir then set the remaining values to zero.
27	HSAR(L,K,J)	Elevation (units of FT above Mean Sea Level) at which reservoir surface area SAR(L,K,J) is defined. Values should be read in from the top of the reservoir to the bottom of the reservoir. L index goes from 1 to 8; if less than 8 values are needed to describe the reservoir then set the remaining values to zero.
28**	LAD(K,J)	Reach number corresponding to location of dam K.
	HDD(K,J)	Elevation (units of FT above Mean Sea Level) of top of dam. If the default value is specified by setting this value to 0.00 then the value printed will be the default value actually used in the calculations and the value punched will be 0.00. <u>1/</u>
	CLL(K,J)	Length (units of FT) of the dam crest less the length of the uncontrolled spillway and gates. If CLL(K,J) is entered as a negative value, the length of the dam crest is variable with elevation and will be specified later as DG-30 and DG-31.
	CDOD(K,J)	Discharge coefficient for uncontrolled weir flow over the top of the dam (2.6-3.1). If the default value is specified by setting this value to 0.00 then the value printed will be the default value actually used in the calculations and the value punched will be 0.00. <u>1/</u>
	QTD(K,J)	Discharge (units of CFS) through turbines. This flow is assumed constant from start of computations until the dam is 1/4 breached; thereafter, QTD(K,J) is assumed to linearly decrease to zero when 1/2 breached; QTD(K,J) may also be considered leaking or constant spillway flow. If this flow is time-dependent, QTD(K,J) is entered with any negative value and the time series for QTD(K,J) is specified later on DG-32 and DG-33.
	ICHAN(K,J)	Parameter indicating if channel conditions at dam K will switch from manual control (e.g., lock and dam controlled by the lockmaster) to

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		channel control (i.e., unsteady flow conditions). If no manual control then set ICHAN(K,J)=0; if channel control switch is allowed then set ICHAN(K,J)=1.
		If dam is represented by a rating curve only then set all values in DG-28 to zero except LAD(K,J).
29**	ICG(K,J)	Parameter indicating type of movable gate structure. If ICG(K,J)=0, no movable gates exist; if ICG(K,J)=1, movable gates exist using an average gate opening; if ICG(K,J)=2, multiple movable gates exist with independent gate openings.
	HSPD(K,J)	Elevation (units of FT above Mean Sea Level) of uncontrolled spillway crest. If no spillway exists then set HSPD(K,J)=0. If the default value is specified by setting this value to 0.00 then the value printed will be the default value actually used in the calculations and the value punched will be 0.00. <u>1/</u>
	SPL(K,J)	Crest length (units of FT) of uncontrolled spillway. If no spillway exists, let SPL(K,J)=0.
	CSD(K,J)	Discharge coefficient of uncontrolled spillway (2.6-3.2). If CSD(K,J)<0, the failure starts in the spillway at its crest and failure is confined to a length of the spillway. If no spillway exists then set CSD(K,J)=0. If spillway is represented by an empirical rating curve then set CSD(K,J)=0 and HSPD(K,J)>0. Note that only one empirical rating is allowed at the dam. If several rating curves exist at the dam, they should be combined and entered as one rating curve.
	HGTD(K,J)	Elevation (units of FT above Mean Sea Level) of center of gate openings for average moveable gates. If the default value is specified by setting this value to 0.00 then the value printed will be the default value actually used in the calculations and the value punched will be 0.00. <u>1/</u>
	CGD(K,J)	Discharge coefficient for gate flow (0.60-0.80) times the area of the gates (units of FT ²). If no gate exists then set CGD(K,J)=0. If gates are represented by an empirical

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
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rating curve then set CGD(K,J)=0. Note that only one empirical rating is allowed at the dam. If several rating curves exist at the dam, they should be combined and entered as one rating curve. If the average moveable gate option is used and submergence effects are expected, an empirical rating curve with built-in submergence should be used.

If dam is represented by a rating curve only then set all values in DG-29 to zero except HSPD(K,J).

Skip DG-30 and DG-31 if the dam crest length is constant (CLL(K,J) is greater than or equal to zero, DG-28).

30	HCRESL(L,K,J)	Elevation (units of FT above Mean Sea Level) associated with variable length of dam crest, CRESL(L,K,J), for dam. Values should be read-in starting at the minimal crest elevation to the maximum elevation. L index goes from 1 to 8; if less than 8 values are needed to describe the dam crest then set the remaining values to zero.
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31	CRESL(L,K,J)	Variable length (units of FT) of dam crest for a given elevation, HCRESL(L,K,J). L index goes from 1 to 8; if less than 8 values are needed to describe the dam crest then set the remaining values to zero.
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Skip DG-32 through DG-39 if running in NWSRFS (not stand-alone mode).

Skip DG-32 and DG-33 if the turbine flow is constant (QTD(K,J), DG-28, is greater than or equal to 0).

32	QTT(L,K,J)	Variable discharge (units of CFS) through the turbines; this flow is time dependent. L index goes from 1 to NU (DG-2).
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33	TQT(L,K,J)	Time (units of HR) associated with discharge through turbines, QTT(L,K,J). L index goes from 1 to NU (DG-2).
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Skip DG-34 and DG-35 if no rating curve is generated for the spillway or gate structure (KRCHT(K,J), DG-20,) is not equal to 11,21,12).

34	RHI(L,K,J)	Head (units of FT) above spillway crest or gate center. Head is associated with spillway or gate flow, RQI(L,K,J), in rating curve. L index goes from 1 to 8; if less than 8 values
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<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		are needed to describe the rating curve then set the remaining values to zero.
35	RQI(L,K,J)	Discharge (units of CFS) of spillway or gate rating curve corresponding to RHI(L,K,J). L index goes from 1 to 8; if less than 8 values are needed to describe the rating curve then set the remaining values to zero.

Skip DG-36 through DG-39 if no multiple movable gates (KRCHT(K,J) is not equal to 14).

36	NG(K,J)	Number of movable gates in dam K.
37	GSIL(L,K,J)	Elevation (units of FT above Mean Sea Level) of the bottom of gate L.
	GWID(L,K,J)	Width of gate opening on gate L.
38	TGHT(I,L,K,J)	Time (units of HR) associated with gate opening GHT(L,K,J). I index goes from 1 to KCG (DG-3).
39	GHT(I,L,K,J)	Distance (units of FT) from bottom of gate to gate sill, GSIL(I,L,K,J). This distance is time dependent and is associated with the time array TGHT(I,L,K,J); I index goes from 1 to KCG.

Repeat DG-37 through DG-39 for each movable gate (L=1,NG(K,J)).

Skip DG-40 through DG-43 if internal boundary is not a lock and dam (KRCHT(K,J), DG-20, is not equal to 28).

40	PTAR(K,J)	Elevation (units of FT above Mean Sea Level) of water surface in headwater pool at upstream face of lock and dam; this elevation is considered the target pool elevation; the lock-master controls the flow through the dam via gates to maintain the pool elevation at this target elevation.
41	CHTW(K,J)	Elevation (units of FT above Mean Sea Level) of water surface in tailwater pool at downstream face of lock and dam; this elevation is considered the elevation at which the lock-master can no longer control the flow through the dam and the flow becomes channel controlled; usually this elevation will be equal to or slightly less than the target pool elevation.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
Skip DG-42 and DG-43 if running in NWSRFS (not in stand-alone mode).		
42	POLH(L,K,J)	Target pool elevation (same as PTAR(K,J)) for each time step; if POLH(L,K,J)=0.0 is read-in, then PTAR(K,J) is used for POLH(L,K,J). L index goes from 1 to NU (DG-2). These elevations are associated with the inflow hydrograph time array.
Skip DG-43 if lock and dam will not be manually controlled (ICHAN(K,J)=0, DG-28).		
43	ITWT(L,K,J)	Parameter indicating if gates control the flow; if ITWT(L,K,J)=0, flow is controlled by the gates; if ITWT(L,K,J)=1, flow is not controlled by the gates, e.g., the entire dam is removed as in the case of the low lift dams on the lower Ohio River and the flow becomes channel controlled. L index goes from 1 to NU (DG-2). These gate control switches are associated with the inflow hydrograph time array.
Skip DG-44 through DG-46 if internal boundary is not a bridge (KRCHT(K,J) is not equal to 35).		
44	LAD(K,J)	Reach number corresponding to location of bridge K.
	EMBEL2(K,J)	Crest elevation (units of FT above Mean Sea Level) of uppermost portion of road embankment.
	EMBW2(K,J)	Crest length (units of FT) of uppermost portion of road embankment (including bridge opening) measured across valley and perpendicular to flow.
	EMBEL1(K,J)	Crest elevation (units of FT above Mean Sea Level) of lower portion (emergency overflow) of road embankment. If nonexistent then set EMBEL1(K,J)=0.
	EMBW1(K,J)	Crest length (units of FT) of lower portion of road embankment measured across valley and perpendicular to flow. If nonexistent then set EMBW1(K,J)=0.
	BRGW(K,J)	Width (units of FT) of top of road embankment as measured parallel to flow.
	CDBRG(K,J)	Coefficient of discharge of flow through bridge opening (see Chow, Open Channel

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		Hydraulics, pages 476-490).
45	BRGHS(L,K,J)	Elevations (units of FT above Mean Sea Level) associated with widths of bridge opening; the brige opening should be closed by setting the last BRGHS(L,K,J) slightly higher (say 0.1 FT.) than the previous value; start at invert and proceed upwards. L index goes from 1 to 8; if less than 8 values are needed to describe the bridge opening then set the remaining values to zero.
46	BRGBS(L,K,J)	Width (units of FT) associated with BRGHS(L,K,J) elevation of bridge opening; the bridge opening should be closed by setting the last BRGBS(L,K,J)=0; start at invert and proceed upwards. L index goes from 1 to 8; if less than 8 values are needed to describe the bridge opening then set the remaining values to zero.
Skip DG-47 if internal boundary is not a dam or a bridge.		
47**	TFH(K,J)	Time (units of HR) from beginning of breach formation until it reaches its maximum size in dam/bridge K.
	DTHDB(K,J)	Computational time step (units of HR) to be used after failure of dam/bridge K. If DTHDB(K,J)=0, the time step size will be computed as TFH(K,J)/MDT; if multiple dams/bridges have failed, the smallest time step will be used during computations.
	HFDD(K,J)	Elevation (units of FT) of water when failure of dam/embankment K commences. If HFDD(K,J)<0, failure commences at time equal to the absolute value of HFDD(K,J) (units of HR).
	BBD(K,J)	Final (maximum) width (units of FT) of bottom of breach.
	ZBCH(K,J)	Side slope (1 vertical : ZBCH(K,J) horizontal) of breach.
	YBMIN(K,J)	Lowest elevation (units of FT above Mean Sea Level) that bottom of breach reaches.
	BREXP(K,J)	Exponent used in development of breach. Varies from 1 to 4; a typical value is 1.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	CPIP(K,J)	Centerline elevation (units of FT above Mean Sea Level) of piping breach. If breach is overtopping then set CPIP(K,J)=0.

Repeat DG-26 through DG-47 for each dam/bridge on river J, K=1,NDB where NDB is the number of dams/bridges; then repeat again for each river (J=1,JN).

Skip DG-48 and DG-49 if NQL(J) is less than or equal to 0.

48	LQ1(K,J)	Sequence number of upstream cross section with lateral inflow. LQ1(K,J) must be placed in columns 1-10.
	STNAME(K,J)	Time series identifier for cross section with lateral flow. STNAME(K,J) may be up to 8 characters long and it must begin in column 13. Omit this field if running in stand-alone mode.
	DTYPE(K,J)	Time series data type for cross section with lateral flow. DTYPE(K,J) may be up to 4 characters long and it must begin in column 22. Omit this field if running in stand-alone mode.

Skip DG-49 if running in NWSRFS (not stand-alone mode).

49	QL(L,K,J)	Lateral inflow at cross section LQ1(K,J). L index goes from 1 to NU. This hydrograph is associated with the inflow hydrograph time array.
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Repeat DG-48 and DG-49 for each lateral flow (K=1,NQL(J), DG-13); then repeat again for each river (J=1,JN).

Skip DG-50 through DG-53 if NGAGE(J)=0 (DG-13).

50	NGS(K,J)	Sequence number of cross section that is an observed/plotting station. NGS(K,J) must be placed in columns 1-10.
	GZ(K,J)	Gage correction to convert observed stages to mean sea level datum. GZ(K,J) must be placed in columns 11-20. Omit this field if KPL=2 (DG-4) or IOBS is less than or equal to 0 (DG-5).
	STNAME(K,J)	Time series identifier for cross section where observed data will be available or where plotting will be done. STNAME(K,J) may be up to 8 characters long and it must begin

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		in column 23 (it must begin in column 13 if GZ(K,J) is omitted). If running in stand-alone mode, STNAME(K,J) may be up to 20 characters long.
	DTYPE(K,J)	Time series data type for cross section where observed data will be available or where plotting will be done. DTYPE(K,J) may be up to 4 characters long and it must begin in column 32 (it must begin in column 22 if GZ(K,J) is omitted). Omit this field if running in stand-alone mode.
Skip DG-51 if running in NWSRFS (not stand-alone mode) or IOBS (DG-5) is less than or equal to 0.		
51	STT(L,K,J)	Observed stage or discharge time series at cross section NGS(K,J). L index goes from 1 to NU. The time array associated with this hydrograph is the same as for the inflow hydrograph.
Skip DG-52 & DG-53 if KPL (DG-4) is not equal to 3 or IOBS (DG-5) is less than or equal to 0.		
Skip DG-52 if running in stand-alone mode (not a part of NWSRFS).		
52	STNAME(K,J)	Time series identifier for cross section where observed discharges will be available or where discharges will be plotted. STNAME(K,J) may be up to 8 characters long and it must begin in column 3.
	DTYPE(K,J)	Time series data type for cross section where observed discharges will be available or where discharges will be plotted. DTYPE(K,J) may be up to 4 characters long and it must begin in column 12. Omit this field if running in stand-alone mode.
Skip DG-53 if running in NWSRFS (not stand-alone mode).		
53	STQ(L,K,J)	Observed discharge time series at cross section NGS(K,J), DG-50. L index goes from 1 to NU (DG-2). The time array associated with this hydrograph is the same as for the inflow hydrograph.
Repeat DG-50 through DG-53 for each gaging station (K=1,NGAGE(J), DG-13); then repeat the group for each river (J=1,JN).		
Skip DG-54 if NSTR(J)=0 (DG-13) or if running in stand-alone mode		

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
(not a part of NWSRFS).		
54	NST(K,J)	Sequence number of upstream cross section with an output time series. NST(K,J) must be placed in columns 1-10.
	STNAME(K,J)	Time series identifier for cross section with output time series. STNAME(K,J) may be up to 8 characters long and it must begin in column 13.
	DTYPE(K,J)	Time series data type for cross section with output time series. DTYPE(K,J) may be up to 4 characters long and it must begin in column 22.
	GZO(K,J)	Gage correction to convert output water surface elevations to stages. GZO(K,J) must be placed in columns 26-36. If the output time series is not stage then set GZO(K,J)=0.

Repeat DG-54 for each output time series (K=1,NSTR(J), DG-13); then repeat the group for each river (J=1,JN).

Skip DG-55 if IOBS is greater than or equal to 0.

55	TPG(J)	Time (units of HR) from initial steady flow to peak of specified upstream boundary hydrograph (used in mathematical function describing the hydrograph).
	RHO(J)	Ratio of peak value of specified hydrograph to initial value of the hydrograph.
	GAMA(J)	Ratio of time TG to TPG(J) where TG is time from initial steady flow to center of gravity of the specified hydrograph. GAMA(J) must be >1.
	YQI(J)	Initial steady discharge (units of CFS) or water surface elevation (units of FT above Mean Sea Level) at the upstream boundary.

Repeat DG-55 for each river (J=1,JN).

Skip DG-56 through DG-58 if KU(J)>2 (DG-13).

Skip DG-56 and DG-57 if running in NWSRFS (not stand-alone mode).

56	ST1(L,J)	Stages (units of FT) or discharges (units of CFS) at upstream boundary of river J. L index goes from 1 to NU (DG-2).
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<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
Skip DG-57 if DTHYD>0 (DG-1).		
57*	T1(L,J)	Time array associated with upstream hydrograph ST1(L,J). L index goes from 1 to NU (DG-2).
Skip DG-58 if running in stand-alone mode (not a part of NWSRFS) and KU(J)=2 (DG-13).		
58*	STM(J)	Minimum stage (units of FT) or discharge (units of CFS) allowed at the upstream boundary. STM(J) must be placed in columns 1-10. Omit this parameter if running in stand-alone mode (not a part of NWSRFS).
	GZ1(J)	Gage correction to convert upstream stages to mean sea level datum. GZ1(J) must be placed in columns 11-20. Omit this parameter if KU(J)=2 (DG-13).
	STNAME(J)	Time series identifier for stages (units of FT) or discharges (units of CFS) at the upstream boundary. STNAME(J) may be up to 8 characters long and it must begin in column 23 (it must begin in column 13 if GZ1(J) is omitted). Omit this parameter if running in stand-alone mode (not a part of NWSRFS).
	DTYPE(J)	Time series data type for observed stages (units of FT) or discharges (units of CFS) at the upstream boundary. DTYPE(J) may be up to 4 characters long and it must begin in column 32 (it must begin in column 22 if GZ1(J) is omitted). Omit this parameter if running in stand-alone mode (not a part of NWSRFS).
Repeat DG-56 through DG-58 for each river (J=1,JN).		
Skip DG-59 through DG-65 if KD(J)=0 (DG-13).		
Skip DG-59 through DG-63 if KD(1)>2 (DG-13).		
Skip DG-59 if running in NWSRFS (not stand-alone mode).		
59	STN(K,1)	Observed stages (KD(1)=1) or discharges (KD(1)=2) at downstream boundary of main river. K index goes from 1 to NU (DG-2).
Skip DG-60 if running in stand-alone mode (not a part of NWSRFS) and KD(1) is not equal to 1 or 3 (DG-13).		
60	GZN	Gage correction (units of FT above Mean Sea

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		Level) to convert downstream stages to mean sea level datum. GZN must be placed in columns 1-10. Omit this field if KD(1) is not equal to 1 or 3(DG-13).
	STNAME	Time series identifier for stages (KD(1)=1, DG-13) or discharges (KD(1)=2, DG-13) at downstream boundary of main river. STNAME may be up to 8 characters long and it must begin in column 13 (it must begin in column 1 if GZN is omitted). Omit this parameter if running in stand-alone mode (not a part of NWSRFS).
	DTYPE	Time series data type for stages (units of FT) or discharges (units of CFS) at the downstream boundary. DTYPE may be up to 4 characters long and it must begin in column 22 (it must begin in column 12 if GZN is omitted). Omit this parameter if running in stand-alone mode (not a part of NWSRFS).
Skip DG-61 through DG-63 if running in stand-alone mode (not a part of NWSRFS).		
Skip DG-61 and DG-62 if the NOS tide is not used for the downstream boundary (KD(1), DG-13, is not equal to 0).		
61	STNAME	Time series identifier for NOS tide at the downstream boundary. STNAME may be up to 8 characters long and it must begin in column 3.
	DTYPE	Time series data type for NOS at the downstream boundary. DTYPE may be up to 4 characters long and it must begin in column 13.
62	STNAME	Time series identifier for the adjusted tide at the downstream boundary. STNAME may be up to 8 characters long and it must begin in column 3.
	DTYPE	Time series data type for the adjusted tide at the downstream boundary. DTYPE may be up to 4 characters long and it must begin in column 13.
Skip DG-63 if computed hydrographs are not adjusted (IOBS, DG-5, is less than 2).		
63	STNAME(K,J)	Time series identifier for cross section

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		location where the computed stage or discharge hydrograph will be adjusted based on observed data. STNAME may be up to 8 characters long and it must begin in column 3.
	DTYPE(K,J)	Time series data type for cross section location where the computed stage or discharge hydrograph will be adjusted based on observed data. DTYPE may be up to 4 characters long and it must begin in column 13.
Repeat DG-63 for each gaging station (K=1,NGAGE(J), DG-13); then repeat the group for each river (J=1,JN).		
Skip DG-64 if KD(1), DG-13, is not equal to 0, or if running in stand-alone mode (not a part of NWSRFS).		
Skip DG-64 through DG-66 if NYQD=0 (DG-3) OR KD(1) is not equal to 3 (DG-13).		
Skip DG-64 if running in stand-alone mode (not a part of NWSRFS).		
64	STNAME	Rating curve identifier for cross section at downstream boundary on main river. STNAME may be up to 8 characters long and it must begin in column 1.
Skip DG-65 and DG-66 if running in NWSRFS (not stand-alone mode).		
65	YQD(K)	Stages (units of FT) used to define the empirical rating curve at the downstream boundary on the main river. K goes from 1 to NYQD.
66	QYQD(K)	Discharge (units of CFS) used to define the empirical rating curve at the downstream boundary on the main river. K goes from 1 to NYQD.
Skip DG-67 if KD(1) is not equal to 5.		
67	SLFI(1)	Bed/initial water surface slope (units of FT /FT) of the main river. This slope is used to generate the single-valued rating curve at the downstream boundary.
Skip DG-68 and DG-69 if internal boundary is not a lock and dam (KRCHT(K,J) is not equal to 28, DG-20) or if running in stand-alone mode (not a part of NWSRFS).		

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
68	STNAME(K,J)	Time series identifier for cross section with target pool elevations. STNAME(K,J) may be up to 8 characters long and it must begin in column 3.
	DTYPE(K,J)	Time series data type for cross section with target pool elevations. DTYPE(K,J) may be up to 4 characters long and it must begin in column 12.
Skip DG-69 if lock and dam will never switch to channel control (ICHAN(K,J)=0, DG-28)		
69	STNAME(K,J)	Time series identifier for cross section with gate control switches. STNAME(K,J) may be up to 8 characters long and it must begin in column 3.
	DTYPE(K,J)	Time series data type for cross section with gate control switches. DTYPE(K,J) may be up to 4 characters long and it must begin in column 12.
Repeat DG-68 and DG-69 for each lock and dam (KRCHT(K,J)=28, DG-20); then repeat the group for each river (J=1,JN).		
Skip DG-70 through DG-75 if NP is not equal to -4.		
70	IFXC(I,J)	Parameter indicating if cross section has special properties when CALXS option is used. If no special properties, IFXC(I,J)=0; if actual section is to be read in, IFXC(I,J)=1; I index goes from 1 to NBT(J), DG-12.
71	HSC(J)	Invert elevation (units of FT) at the most upstream cross section on river J.
72	KAM	Parameter indicating the method for reading in cross sections in the calibration reach. If KAM=0, cross sections are described as topwidth versus depth (B versus Y) at key points in the cross section (see Figure 1 [Bookmark]); if KAM=1, cross sections are described as the power function $B=kY^m$ where m is a shape factor and k is a scaling factor (see Figure 2 [Bookmark]).
	CHNMN(I,J)	The minimum acceptable Manning n value computed during automatic calibration for calibration reach I. The default value is 0.013.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	CHNMX(I,J)	The maximum acceptable value of Manning n value computed during automatic calibration for calibration reach I. The default value is 0.25.
	SXS	Average channel bottom slope (units of FT/MI) along calibration reach I.
Skip DG-73 if KAM=0.		
73	FKC(I,J)	Scaling parameter of the channel in-bank portion of cross section in calibration reach I described in power function.
	FMC(I,J)	Shape factor for the channel in-bank portion of cross section in calibration reach I described in power function.
	FKF(I,J)	Scaling parameter of floodplain portion of cross section in calibration reach I described in power function.
	FMF(I,J)	Shape factor for floodplain portion of cross section in calibration reach I described in power function.
	FKO(I,J)	Scaling parameter of dead storage (inactive) portion of cross section in calibration reach I described in power function.
	FMO(I,J)	Shape factor for dead storage (inactive) portion of cross section in calibration reach I described in power function.
	HB	Depth (units of FT) of cross section at top of bank.
	HF	Depth (units of FT) of cross section at top of floodplain.
Skip DG-74 and DG-75 if KAM=1.		
74	B1	Active top width (units of FT) of typical cross section in calibration reach I at depth Y1 (half of channel depth).
	B2	Active top width (units of FT) of typical cross section in calibration reach I at depth Y2 (top of bank).
	B3	Active top width (units of FT) of typical cross section in calibration reach I at depth

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		Y3 (midpoint of floodplain). Enter zero if no floodplain.
	B4	Active top width (units of FT) of typical cross section in calibration reach I at depth Y4 (maximum flood depth). Enter zero if no floodplain.
	B5	Dead storage (inactive) top width (units of FT) of typical cross section in calibration reach I at depth Y3. Enter zero if no inactive storage.
	B6	Dead storage (inactive) top width (units of FT) of typical cross section in calibration reach I at depth Y4. Enter zero if no inactive storage.
75	Y1	Depth (units of FT) of typical cross section in calibration reach I at mid-point between the invert and top of bank.
	Y2	Depth (units of FT) of typical cross section in calibration reach I at top of bank.
	Y3	Depth (units of FT) measured from invert of typical cross section in calibration reach I to midpoint between the top of bank and estimated maximum flood elevation.
	Y4	Depth (units of FT) of typical cross section in calibration reach I at an estimated maximum flood elevation.
Repeat DG-72 through DG-75 for each calibration reach (I=1,NGAGE(J)-1).		
76*	FLST(I,J)	Elevation (units of FT above Mean Sea Level) at which flooding commences. If no flood stage, enter zero.
	YDI(I,J)	Initial water surface elevation (units of FT above Mean Sea Level) at cross section I. If steady state conditions exist, the YDI value at the downstream location of the main river and pool levels behind dams must be read in (all other values are entered as zero) and the model will do backwater computations; otherwise, all values are read in. Omit this parameter if running in NWSRFS (not stand-alone mode).

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
	QDI(I,J)	Initial discharge (units of CFS) at cross section I. If steady state conditions exist, all QDI values are read in as zero and the QDI values are generated by summation of flows from upstream to downstream. If KU(J) is not equal to 2, the upstream discharge (QDI(I,J)) must be read in. If unsteady-state condition exists, all QDI values are read in. Omit this parameter if running in NWSRFS (not stand-alone mode).
	AS(1,I,J)	Active channel cross-sectional area (units of FT ²) below the lowest HS elevation in cross section I.
	XLAT(I,J)	Latitude of the river centerline as it crosses the channel bottom. If no flood mapping (NMAP=0), omit this field.
	XLON(I,J)	Longitude of the river centerline as it crosses the channel bottom. If no flood mapping (NMAP=0), omit this field.

Skip DG-77 through DG-83 if NP=-4 and IFXC(I,J)=0.

77*	HS(L,I,J)	Elevation (units of FT above Mean Sea Level) corresponding to each top width BS(L,I,J). Elevations are entered from the bottom of the cross section upward; L index goes from 1 to NCS.
78*	BS(L,I,J)	Top width (units of FT) of active flow portion of channel/valley cross section corresponding to each elevation HS(L,I,J). L index goes from 1 to NCS.

Skip DG-79 and DG-80 if KFLP=0.

79	BSL(L,I,J)	Top width (units of FT) of active flow portion of left floodplain corresponding to each elevation HS(L,I,J). L index goes from 1 to NCS.
80	BSR(L,I,J)	Top width (units of FT) of active flow portion of right floodplain corresponding to each elevation HS(L,I,J). L index goes from 1 to NCS.

Skip DG-81 and DG-82 if KFLP is less than or equal to 1 .

81	HKC(L,I,J)	Elevation (units of FT above Mean Sea Level) corresponding to the conveyance QKC(L,I,J). L
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<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		index goes from 1 to KFLP.
82	QKC(L,I,J)	Conveyance corresponding to elevation HKC(L,I,J). I index goes from 1 to KFLP.
83*	BSS(L,I,J)	Top width (units of FT) of dead storage (inactive) portion of channel/valley cross section corresponding to each elevation HS(L,I,J). K index goes from 1 to NCS; if no inactive storage exists, enter zero.

Repeat DG-76 through DG-83 for each cross section (I=1,NBT(J)).

Repeat DG-70 through DG-83 for each river (J=1,JN).

Skip DG-84 if KFLP is not equal to 1.

84	SNM(L,I,J)	Sinuosity coefficient (channel flow-path length/floodplain flow-path length corresponding to each elevation HS(L,I,J). L index goes from 1 to NCS.
----	------------	--

Repeat DG-84 for all reaches (I=1,NBT(J)-1).

85*	FKEC(I,J)	Expansion or contraction coefficients. Expansion coefficients vary from -.05 to -.75 and contraction coefficients vary from +.10 to +.40, the larger values are associated with very abrupt changes in cross section along the river; if expansion/contraction is negligible then set FKEC(I,J)=0. I index goes from 1 to NBT(J)-1.
-----	-----------	---

86*	NCM(I,J)	Station number of upstream-most station in subreach that has the same Manning n. I index goes from 1 to NRCM1(J).
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87*	CM(L,I,J)	Manning n corresponding to each YQCM(L,I,J) value. L index goes from 1 to NQCM(J); if NQCM(J)=0, Manning n values are treated as in the DAMBRK program where Manning n is a function of the average elevation between two cross sections and L index goes from 1 to NCS.
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Skip DG-88 and DG-89 if KFLP=0.

88	CML(L,I,J)	Manning n corresponding to each YQCM(L,I,J) value for left floodplain. L index goes from 1 to NQCM(J); the same rules apply for NQCM(J) as were previously stated in DG-84.
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89	CMR(L,I,J)	Manning n corresponding to each YQCM(L,I,J) value for right floodplain. L index goes from 1 to NQCM(J); the same rules apply for NQCM(J) as were previously stated in DG-84.
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Skip DG-90 if NQCM(J)=0.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
90	YQCM(L,I,J)	Water surface elevation (units of FT above Mean Sea Level) or discharges (units of CFS) associated with Manning n. L index goes from 1 to NQCM(J).

Repeat DG-87 through DG-90 for each Manning reach (I=1,NRCM1(J)).

Repeat DG-84 through DG-90 for each river (J=1,JN).

Skip DG-88 through DG-94 if DG-66 if running in stand-alone mode (not a part of NWSRFS).

Skip DG-88-90 if IOBS (DG-5) is not equal to 3

88	NSLC(J)	Total number of slices used to adjust the computed time series. J index goes from 1 to JN.
----	---------	--

89	NQSL(J)	Parameter indicating adjustment statistics are a function of water surface elevation (NQSL(J)=0), or discharge (NQSL(J)=1). J index goes from 1 to JN.
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90	SLICE(L,K,J)	Stage (units of FT) or discharge (units of CFS) range into which the statistics lie. A hydrograph will be divided into NSLC(J) elevation or discharge ranges (slices) and adjusted based on the root mean square error and bias. L index goes from 1 to NSLC(J).
----	--------------	--

Skip DG-91 through DG-94 if IOBS, DG-5, is less than 2.

91	FRMSO(L,K,J)	Root mean square error (rms) on the falling limb of the hydrograph within each slice. This value is used when no observed data exists in the slice for the current runtime. If FRMSO (L,K,J)=0, no adjustment is made to the computed stage. L index goes from 1 to NSLC(J).
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92	FBIASO(L,K,J)	Bias associated with FRMSO (L,K,J). L index goes from 1 to NSLC(J).
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93	RRMSO(L,K,J)	Root mean square error (rms) on the rising limb of hte hydrograph within each slice. This value is used when no observed data exists in the slice for the current runtime. If RRMSO (L,K,J)=0, no adjustment is made to the computed stage. L index goes from 1 to NSLC(J).
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94	RBIASO(L,K,J)	Bias associated with RRMSO (L,K,J). L index goes from 1 to NSLC(J).
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Repeat DG-90 through DG-94 for each adjusted time series (K=1, NGAG(J), DG-13); then repeat the group for each river (J=1,JN).

Skip DG-95 through DG-99 if ICOND=1 (DG-2).

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
95	YDI(I,J)	Initial water surface elevation referenced to msl (units of FT) at each cross section. Each field represents a cross section. I=1,NBT(J) (DG-12). If all fields are left blank, the program will generate the YDI's via linear interpolation between gaging stations (this is allowed when gaging stations exist at the upstream extremities of all rivers and the downstream extremity of the main stem). If the upstream extremity of each river does not have an observed hydrograph, this YDI value must be supplied along with all the blanks for the other YDI's. If all fields are left blank except at the downstream extremity of the main stem river where the actual YDI is read in, the program will generate the YDI's via a solution of the steady flow backwater equation.
Repeat DG-95 for each river (J=1,JN).		
96	QDI(I,J)	Initial discharges (units of CFS) at each cross section. Each field represents a cross section I=1,NB(J) (DG-12). If all fields are left blank except at the upstream extremity of each river, the program will generate the QDI's by summation of the flows from the upstream to downstream boundaries, including tributary inflow to the main stem and lateral inflow occurring along either the main stem or tributaries.
Repeat DG-96 for each river (J=1,JN).		
Skip DG-97 if no lateral flow in the system (NQL(J)=0, DG-13, for all rivers J=1,JN)		
97	QLI(K,J)	Initial lateral flow (units of CFS) for each reach with lateral flow. Each field represents a lateral flow reach. K=1,NQL(J) (DG-13).
Repeat Card number 97 for each river with lateral flow (NQL(J) not equal to 0, J=1,JN)		
Skip DG-98 and DG-99 if no lock and dams in the system (all KRCHT values equal zero, DG-20).		
98	PLTI(K,J)	Initial target pool elevation for each lock and dam. Each field represents a lock and dam, K=1,NUMLAD(J) where NUMLAD(J) is the sum of KRCHT=28, DG-20).
Repeat DG-98 for each river with locks and dams.		
99	IWTI(K,J)	Initial gate control switch for each lock and dam. Each field represents a lock and dam, K=1,NUMLAD(J) where NUMLAD(J) is the sum of

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
		KRCHT=28, DG-20). K=1,NUMLAD(J).
		Repeat DG-99 for each river with locks and dams.
		Skip DG-100 and DG-101 if NFGRF=1 (DG-4).
100*	MESSAGE	40-character message describing the data set for use in FLDGRF.
101*	RIVER(J)	16-character name associated with river J. There is no comment line prior to this data group.
		Repeat DG-101 for each river (J=1,JN).

Notes:

- 1/ The value output from program FCINIT may be different when the parameters are output with commands PRINTSEGS and PUNCHSEGS. This occurs when the user enters a zero instead of specifying a value which indicates the program should use default values. For example:

```
SEGMENT INPUT:
DAM AT OREGON CITY USING A RATING (ORCF)
 2  0.0  0.0  0.0  0.0  0
DAM INFO
 0  0  0  0  0  0
```

FCINIT Output from PUNCH:

```
LAD      HDD      CLL      CDOD      QTD      ICHAN
 2          0.00      0.00      0.00      3.00      0.00      0
ICG      HSPD      SPL      CSD      HGTD      CGD
 0          0.00      0.00      0.00      0.00      0.00      0.00
```

FCINIT Output from PRINT:

```
LAD      HDD      CLL      CDOD      QTD      ICHAN
 2          100000.00      0.00      0.00      3.00      0.00      0
ICG      HSPD      SPL      CSD      HGTD      CGD
 0          100000.00      0.00      0.00      0.00      100000.00      0.00
```

In this example the HDD (Elevation of the top of dam) variable PUNCH output will contain the value of 0.00 which indicates a default value should be used. The punch output values match the original Segment input allowing the punch output to be used to redefine the Segment. However the PRINT output contains the value which is actually used in the calculations which is 100000.00 (FT above MSL). This allows the user to know exactly what values are being used in the calculations.

Program IFP uses the same rules when it outputs the value.

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Sample Input and Output: Sample input is shown in Figure 3 [\[Bookmark\]](#). Sample output from the parameter print routine is shown in Figure 4 [\[Bookmark\]](#). Sample output from the execution routine is shown in Figures 5 through 29 [\[Bookmark\]](#). The information printed in these figures does not necessarily correspond with the input sample.

<u>Data Group</u>	<u>Variable Name</u>	<u>Contents</u>
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Execution Routine Print Output: The execution print output for Operation FLDWAV is controlled by three input parameters on Card 5:

- JNK (how much information is to be generated)
- KPL (whether or not plots and statistical information is to be generated)
- KPL2 (whether or not observed data are available for plotting or generating statistical information)

Possible output available is:

1. No output - JNK=0, KPL=0.
2. Computed stage plots - JNK=0, KPL=0, KPL2=0.
3. Computed discharge plots - JNK=0, KPL=2, KPL2=0.
4. Plots of observed and computed stages and summary of statistics (Root mean square error and bias error) - JNK=0, KPL=1, KPL2=1.
5. Plots of observed and computed discharges and summary of statistics JNK=0, KPL=2, KPL2=1.
6. A table showing the time step (HR), current time (HR), number of iterations for each river, cross section location (MI), Manning's n, water surface elevation (FT above Mean Sea Level), depth (FT), Froude number, velocity (FT/SEC), discharge (1000 CFS), levee flow (1000 CFS), active flow area (1000 FT2) and total topwidth (FT) at even time increments specified by the KITPR parameter on Card 2, plus any of the plot options I (1) through (5). This option also prints the total number of iterations needed for each river when using the Newton-Raphson Iteration technique, as well as more detail statistical information - JNK=1, KPL=1 or 2, KPL2=0 or 1.

There is no default printer option. JNK, KPL, KPL2 and KITPR must be supplied by the user.

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Error and Warning Messages: Error and warning messages generated by this Operation and the corrective measures to take when they occur are as follows:

A. Messages that can occur during setup.

1. ****ERROR**** NUMBER OF OPERATIONS IN THIS SEGMENT (XXXXXXXX) EXCEEDS THE MAXIMUM NUMBER OF OPERATIONS ALLOWED:YY

Action: If the program is for calibration, then increase the size of the array. If the program is operational, then call your Focal POINT.

2. ****ERROR**** OUTPUT TIME SERIES INTERVAL(DTOUT=(XX)) MUST BE GREATER THAN OR EQUAL TO THE COMPUTATIONAL TIME STEP(DTHII=(YYY.YY))

Action: Increase the output time series interval to be at least as large as the computational time step.

3. ****ERROR**** OUTPUT TIME SERIES INTERVAL (DTOUT=(XX) IS NOT AN EVEN MULTIPLE OF THE COMPUTATIONAL TIME STEP(DTHII=(YYY.YY))

Action: Change the output time interval such that $DHFO = K * TM$ where K is a positive integer.

4. ****WARNING**** OBSERVED DATA TIME STEP (DTHYD=(XX)) IS NOT AN EVEN INCREMENT OF THE TIME STEP FOR PLOTTING COMPUTED DATA/22X(DTHII=(YYY.YY))2X THEREFORE THE OBSERVED DATA WILL NOT BE PLOTTED.

Action: Verify time steps and if plots are desired make $DHF=K*TM*KITPR$ or $K*DHF=TM*KITPR$ where K is a positive integer.

5. ****ERROR**** AUTOMATIC CALIBRATION CANNOT BE USED WHEN THE INFLOW HYDROGRAPH IS GENERATED (NU=0).

Action: If automatic calibration is desired, then specify inflow hydrograph and set $NU>0$. If automatic calibration is not desired set $NP=0$.

6. ****ERROR**** NUMBER OF COMPUTATIONAL POINTS (NBJ=XXX) EXCEEDS MAXIMUM NUMBER OF COMPUTATIONAL POINTS ALLOWED (NBMAX=YYY). PROGRAM TERMINATED.

Action: Check the number of cross sections on each river (NB(J)) and set NBMAX equal to the largest value.

7. ****ERROR**** NUMBER OF OPERATIONS IN THIS SEGMENT (XXXXXXXX) EXCEEDS THE MAXIMUM NUMBER OF OPERATIONS ALLOWED:YY

Action: If the program is for calibration, then increase the size of the array. If the program is operational, then call your Focal Point.

8. ****ERROR**** OUTPUT TIME SERIES INTERVAL(DTOUT=(XX)) MUST BE GREATER THAN OR EQUAL TO THE COMPUTATIONAL TIME STEP(DTHII=(YYY.YYY))

Action: Increase the output time series interval to be at least as large as the computational time step.

9. ****ERROR**** OUTPUT TIME SERIES INTERVAL (DTOUT=(XX)) IS NOT AN EVEN MULTIPLE OF THE COMPUTATIONAL TIME STEP(DTHII=(YYY.YY))

Action: Change the output time interval such that $DHFO = K * TM$ where K is a positive integer.

10. ****WARNING**** OBSERVED DATA TIME STEP (DTHYD=(XX)) IS NOT AN EVEN INCREMENT OF THE TIME STEP FOR PLOTTING COMPUTED DATA/22X(DTHII=(XXX.XX))2X THEREFORE THE OBSERVED DATA WILL NOT BE PLOTTED.

Action: Verify time steps and if plots are desired make $DHF=K*TM*KITPR$ or $K*DHF=TM*KITPR$ where K is a positive integer.

11. ****ERROR**** AUTOMATIC CALIBRATION CANNOT BE USED WHEN THE INFLOW HYDROGRAPH IS GENERATED (NU=0).

Action: If automatic calibration is desired, then specify inflow hydrograph and set $NU>0$. If automatic calibration is not desired set $NP=0$.

12. ****ERROR**** NUMBER OF COMPUTATIONAL POINTS (NBJ=XXX) EXCEEDS

MAXIMUM NUMBER OF COMPUTATIONAL POINTS ALLOWED (NBMAX=YYY).
PROGRAM TERMINATED.

Action: Check the number of cross sections on each river
(NB(J)) and set NBMAX equal to the largest value.

13. **ERROR** KU VALUE MUST BE EQUAL TO '1' OR '2' KU(X)=Y NOT
ACCEPTED.

Action: Check upstream boundary condition. If necessary,
redefine it to be either stage (KU(J)) or discharge (KU(J)).

14. **WARNING** DOWNSTREAM BOUNDARY ON TRIBUTARIES CAN ONLY BE
STAGE. KD(X)=Y NOT ALLOWED. KD(X) HAS BEEN SET TO '1'.

Action: Unless blocked tributary option is used set KD(J)=1 or
0.

15. **WARNING** THIS IS TO AFFIRM THAT RIVER NO. X IS BLOCKED
(KD(J)=2). THEREFORE NO FLOW FROM THIS TRIBUTARY WILL ENTER THE
MAIN RIVER.

Action: If the blocked tributary option is not desired then set
KD(J)=1 or 0.

16. **ERROR** AUTOMATIC CALIBRATION OPTION CANNOT BE USED ON LEVEE
PROBLEMS.

Action: If automatic calibration option is not desired set
NP=0.

17. **ERROR** MANNING'S N VALUES (CM) CANNOT BE SET TO ZERO WHEN
THE AUTOMATIC CALIBRATION OPTION IS NOT USED.

Action: Read in Manning's n values.

18. **ERROR** KPL MUST BE EQUAL TO '1' OR '2'. XXXXXXXX NOT
ACCEPTED.

Action: Set KPL on Card number 5 to '1' or '2'.

19. **ERROR** STAGE DATA CAN ONLY HAVE UNITS OF M (XXXX) NOT
ACCEPTED.

Action: Check the time series header card and change it if
necessary.

20. **WARNING** MISSING DATA ARE NOT ALLOWED IN AUTOMATIC
CALIBRATION RUN.

Action: Check for missing data points and fill the space with
interpolated values.

21. **WARNING** ON RIVER NO. XXX STATION YYYYYYYY WITH DATA TYPE
ZZZ MAY HAVE MISSING DATA. MISSING DATA POINTS WILL NOT BE
PLOTTED OR INCLUDED IN STATISTICS.

Action: Check missing data points and fill them if these points
are to be plotted and included in statistics.

22. **ERROR** TIME SERIES I.D. 'DUMMY' IS NOT ACCEPTABLE WHEN

AUTOMATIC CALIBRATION OPTION IS NOT USED.

Action: Check the time series identifier and redefine if necessary.

23. **ERROR** TIME SERIES I.D. 'DUMMY' IS NOT ACCEPTABLE FOR OUTPUT TIME SERIES.

Action: Check the time series identifier and redefine it if necessary.

24. **ERROR** THE ALLOWABLE DIMENSIONS FOR OUTPUT TIME SERIES ARE 'XXXX'. 'YYYY' IS NOT ALLOWED.

Action: Check the time series header and redefine it if necessary.

25. **ERROR** GATE CONTROL DATA CAN ONLY HAVE UNITS OF 'M'. '(XXXX)' NOT ACCEPTED.

Action: Check the time series header and redefine it if necessary.

26. **ERROR** GATE CONTROL SWITCHES CAN ONLY HAVE UNITS OF 'INT '. '(XXXX)' NOT ACCEPTED.

Action: Check the time series header and redefine it if necessary.

27. **ERROR** INFLOW HYDROGRAPH CAN BE GENERATED (NU=0) ON A SINGLE CHANNEL ONLY (JN=1). JN=XX NOT ALLOWED. PROGRAM TERMINATED.

Action: If the problem has more than one river then set NU>0 and specify an input time series for each upstream boundary.

28. **ERROR** LEVEE MUST BE PLACED ON THE MAIN CHANNEL; THEREFORE NWJ(1) CANNOT BE EQUAL TO ZERO.

Action. If possible make the channel containing the levee the main stem and let fictitious tributary connect to it.

29. **WARNING** ON RIVER NO.'XXXX'STATION '(XXXXXXXXX), WITH DATA TYPE '(XXXX)' MAY HAVE MISSING DATA. MISSING DATA POINTS WILL BE GIVEN VALUES OF ZERO.

Action: No action necessary.

30. **WARNING** THE TABLE OF TOPWIDTHS VS ELEVATION IS NOT COMPLETE FOR SECTION WWW ON RIVER NO. XX. THE LAST GOOD VALUE IS AT LEVEL YYY WHICH HAS AN ELEVATION OF ZZZZZZZZZZ FEET. AT ELEVATIONS HIGHER THAN THIS, THE MODEL WILL LINEARLY EXTRAPOLATE FROM THE LAST TWO POINTS.

Action: Fill the table with the proper number of values.

31. **WARNING** THE TABLE OF TOPWIDTHS VS ELEVATIONS (INACTIVE) IS NOT COMPLETE FOR SECTION WWW ON RIVER NO. XX. THE LAST GOOD VALUE IS AT LEVEL YYY WHICH HAS AN ELEVATION OF ZZZZZZZZZZ FEET. AT ELEVATIONS HIGHER THAN THIS, THE MODEL WILL LINEARLY EXTRAPOLATE FROM THE LAST TWO POINTS.

Action: Fill the table with the proper number of values.

32. ****WARNING**** ON RIVER NO. XX CROSS SECTION NO. YYY HAS BOTH LATERAL FLOW AND FLOW DIVERSION; THEREFORE ONLY THE LATERAL FLOW WILL BE CONSIDERED IN COMPUTATIONS.

Action: Remove either the lateral flow or the flow diversion.

33. ****WARNING**** NO. OF MANNING N REACHES ARE NOT EQUAL TO NO. OF OBSERVED CROSS SECTIONS LESS ONE, SO COMPUTATIONS CAN NOT BE MADE.

Action: Redefine the number of Manning's n reaches.

34. ****ERROR*** THE ALLOWABLE DIMENSIONS FOR ADJUSTED TIDE TIME SERIES ARE 'XXXX'. 'YYYY' IS NOT ALLOWED.

Action: Check the time series header and redefine it if necessary

35. ****ERROR**** THE ALLOWABLE DIMENSIONS FOR ADJUSTED STAGE TIME SERIES ARE 'XXXX'. 'YYYY' IS NOT ALLOWED.

Action: Check the time series header and redefine it if necessary.

B. Messages that can occur during execution.

None.

C. Messages that can occur during carryover transfer:

1. ****WARNING**** THE JN PARAMETER ON DATA GROUP 2 HAS BEEN CHANGED.

Action: No action necessary

2. ****WARNING**** THE NP PARAMETER ON DATA GROUP 5 HAS BEEN CHANGED.

Action: No action necessary

3. ****WARNING**** THE NCS PARAMETER ON DATA GROUP 4 HAS BEEN CHANGED.

Action: FIND STARTING LOCATIONS OF OLD AND NEW PARAMETERS

4. ****WARNING**** THE SLFI PARAMETER ON DATA GROUP 63 HAS BEEN CHANGED.

Action: No action necessary

5. ****WARNING**** THE SLFI PARAMETER ON DATA GROUP 9 HAS BEEN CHANGED.

Action: No action necessary

6. ****WARNING**** NB(' ,I2, ') (INPUT AND INTERPOLATED -DATA GROUPS 12 AND 19) HAS BEEN CHANGED.

Action: No action necessary

7. ****WARNING**** NQL(' ,XX, ') ON DATA GROUP 13 HAS BEEN CHANGED.

Action: No action necessary

8. **WARNING** LQ1(' ,XX,1H, ,XX, ') ON DATA GROUP 48 HAS BEEN CHANGED.

Action: No action necessary

9. **WARNING** LAD(' ,XX,1H, ,XX, ') ON DATA GROUP 44 HAS BEEN CHANGED.

Action: No action necessary

10. **WARNING** NRCM1(' ,XX, ') ON DATA GROUP 13 HAS BEEN CHANGED.

Action: No action necessary

11. **WARNING** NQCM(' ,XX, ') ON DATA GROUP 13 HAS BEEN CHANGED.

Action: No action necessary

12. **WARNING** NCM(' ,XX,1H, ,XX, ') ON DATA GROUP 80 HAS BEEN CHANGED.

Action: No action necessary

13. **WARNING** THE NQCM(' ,XX, ') PARAMETER ON DATA GROUP 13 HAS BEEN CHANGED.

Action: No action necessary

14. **WARNING** YQCM(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 84 HAS BEEN CHANGED.

Action: No action necessary

15. **WARNING** CM(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 81 HAS BEEN CHANGED.

Action: No action necessary

16. **WARNING** CML(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 82 HAS BEEN CHANGED.

Action: No action necessary

17. **WARNING** CMR(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 83 HAS BEEN CHANGED.

Action: No action necessary

18. **WARNING** NBT(' ,XX, ') ON DATA GROUP NO 12 HAS BEEN CHANGED.

Action: No action necessary

19. **WARNING** XT(' ,XXX, ' , ,XX, ON DATA GROUP NO 18 HAS BEEN CHANGED.

Action: No action necessary

20. **WARNING** NN(' ,XXX, ' , ,XX, ') HAS BEEN CHANGED.

Action: No action necessary

21. **WARNING** DXM(' ,XXX, ', ', XX, ON DATA GROUP NO 19 HAS BEEN CHANGED.

Action: No action necessary

22. **WARNING** X(' ,XX,1H, ,XX, ') ON DATA GROUP 25 HAS BEEN CHANGED.

Action: No action necessary

23. **WARNING** FKEC(' ,XX,1H, ,XX, ') ON DATA GROUP 79 HAS BEEN CHANGED.

Action: No action necessary

24. **WARNING** BS(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 72 HAS BEEN CHANGED.

Action: No action necessary

25. **WARNING** BSS(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 77 HAS BEEN CHANGED.

Action: No action necessary

26. **WARNING** HS(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 27 HAS BEEN CHANGED.

Action: No action necessary

27. **WARNING** AS(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 28 HAS BEEN CHANGED.

Action: No action necessary

28. **WARNING** AS(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 73 HAS BEEN CHANGED.

Action: No action necessary

29. **WARNING** AS(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP 74 HAS BEEN CHANGED.

Action: No action necessary

30. **WARNING** SNM(' ,XX,1H, ,XX,1H, ,XX, ') ON DATA GROUP NO 78 HAS BEEN CHANGED.

Action: No action necessary

[[Next](#)] [[Previous](#)] [[Bookmarks](#)] [[Top](#)]

Carryover Transfer Rules: The following rules apply to Operation FLDWAV during the carryover transfer process:

1. Carryover values are initial water surface elevations and discharges at each cross section, initial lateral flow at each lateral flow point, percent of flow initially diverted from the channel, initial target pool elevations and initial gate control switches.

2. Carryover can be transferred (i.e. existing carryover kept) only if none of the following parameters have been changed from one set of data to the next:

<u>Parameter</u>	<u>Data Group</u>	<u>Parameter</u>	<u>Data Group</u>
NBMAX	-	FKC (I, J)	79
NP	5	*NCSS	-
SLFI	63	NCML	81
NB (J)	-	*CM (L, K, J)	-
NQL (L)	13	*YCQM (L, K, J)	-
LQ (I, J)	48	CML	82
SNM	78	CMR	83
BR	-	*NCS	4
NUMLAD (J)	-	*BS (K, I, J)	72
LAD (L, J)	28	*HS (K, I, J)	71
NRCM1 (J)	13	*AS (K, I, J)	70
NQCM (J)	13	ASL	-
NCM (K, J)	80	BSL	73
X (I, J)	18	*BSS (L, I, J)	77
		*ASS (L, I, J)	-

If any of the above parameter have been changed then no carryover transfer will take place (i.e. the carryover values input during the redefinition will be used). The parameters with the (*) beside them may be changed if the change occurs above the initial conditions (e.g., the cross-section properties may be changed as long as table values below the initial conditions remain the same as before.

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Card Punch Limitations: All integer values and alphanumeric names for this Operation are punched out exactly as they are read in. All real values are punched with an F10.2 format with the following exceptions:

<u>Variable Name</u>	<u>Data Group</u>	<u>Punch Format</u>	
EPSY	1	F10.4	
YQCM	84	F10.0	(for discharge)
		F10.2	(for stage)
CM	81	F10.4	
SO	63	F10.6	
STM	-	F10.0	

Table 1. English/Metric Equivalents

<u>Property</u>	<u>English Unit</u>	<u>Metric Unit</u>	<u>Conversion Factor (English to Metric)</u>
Time	HR	HR	
Length	FT	M	1/3.281
Length	MI	KM	1.6093
Flow	FT ³ /SEC	M ³ /SEC	1/35.32
Area	FT ²	M ²	1/10.765
Surface Area	ACRE	KM ²	1/247.1
Volume	ACFT	10**6 M ³	1/810.833
Weir Coefficient	FT**0.5/SEC	M**0.5/SEC	1/1.811
Unit Weight	LB/FT ³	N/M ³	157.1
Shear Strength	LB/FT ²	N/M ²	47.88
Viscosity (Dynamic)	LB SEC/FT ²	N SEC/M ²	47.88
Manning n	English and Metric are same		

Although the documentation refers to English units only, the metric option is fully functional. This table can be used to determine comparable units and to convert the recommended values to metric units.

Table 2. Description of FLDWAV Intermediate Analysis Output

<u>Output Description</u>	JNK					
	<u>0</u>	<u>1</u>	<u>4</u>	<u>5</u>	<u>9</u>	<u>>9</u>
Input Echo Print and Summary of Array Sizes	X	X	X	X	X	X
Bottom Slope Profile			X	X	X	X
Initial Conditions Summary			X	X	X	X
Initial Conditions/Low Flow Filter				X	X	X
Minimum Dynamic Routing Output			X	X	X	X
Internal Boundary Information			X	X	X	X
Hydraulic Information					X	X
Levee Information				X	X	X
Subcritical/Supercritical Information			X	X	X	X
Nonconvergence Information				X	X	X
Calibration Information	X	X	X	X	X	X
Profile of Crests and Times	X	X	X	X	X	X
Computed WSEL and Discharge Hydrograph Data					X	X
Hydrograph Plot	X	X	X	X	X	X
Dynamic Routing Information at each Iteration						X

The FLDWAV model output is controlled primarily by the JNK parameter. JNK may be assigned values of 1, 4, 5, 9, 10 and 12 where the output becomes more extensive as JNK increases. It is recommended that for most runs, JNK be specified as 4; this output is considered to provide the maximum amount of information for the least number of pages of output. A JNK=1 provides the least amount of output and is intended to be used for obtaining final results to minimize permanent paper or file storage requirements. If the graphical output utility (FLDGRF) is to be used to review the output information, JNK=1 may be used. JNK values greater than 4 are to be used to obtain detailed hydraulic and numerical information for confronting and overcoming numerical difficulties that have caused aborted runs or suspect results. When a user first sets up the problem, it is highly recommended that JNK>=5 be used to ensure that the model is behaving in an acceptable manner for the problem being modeled. In some cases, the hydrographs and boundary information may indicate a successful run, but further inspection of the hydraulic information throughout the routing reach may reveal hidden problems in the data input setup.

Examples of the output shown in Figures 5 through 29. Parameters in bold print are defined. No examples are given for JNK>=10 since this is usually repetitive information per iteration. Generally, output variables are defined categorically with the first or first two letters; i.e., Q is discharge, Y is water surface elevation, X is cross-section distance location, FR is Froude number, T is time, V is velocity, A is wetted cross-sectional area, B is wetted cross-sectional topwidth and CM is Manning n. Also, the J and I counters refer to the river number and cross-section number, respectively.

Table 3. Routing methods and internal boundaries

<u>KRCHT(I,J)</u>	<u>Definitions</u>
0	Implicit Dynamic Routing
1	Implicit (Diffusion) Routing
4	Level Pool Routing
5	Explicit Dynamic Routing (Upwind)
6	Implicit (Local Partial Inertial) Routing
10	Dam
11	Dam + $Q=f(Y)$
21	Dam + $Y=f(Q)$
12	Dam + $Q=f(Y,Y)$
14	Dam + Multiple Movable Gates $C=f(Y,HG,FR)$
15	Dam + Average Movable Gates (Corps of Engineers Type)
28	Lock and Dam
35	Bridge

Variable Definitions

Q = flow
Y = pool elevation
YY = tailwater elevation
HG = centerline of gate
C = gate coefficient
FR = Froude number

Figure 1. Cross section description for calibration option

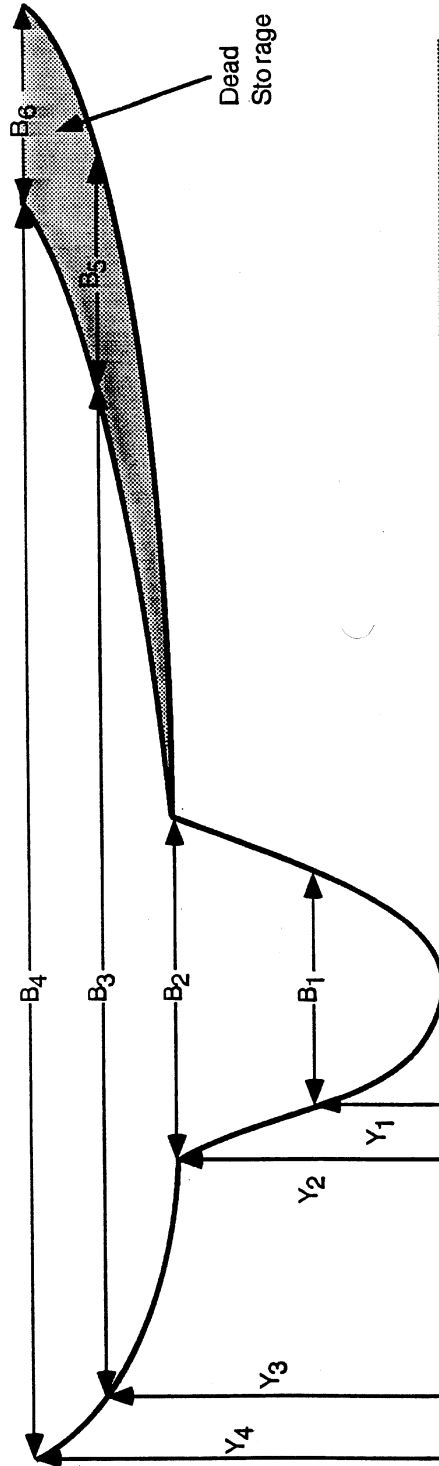


Figure 2. Cross section shapes for power function $B=kY^m$

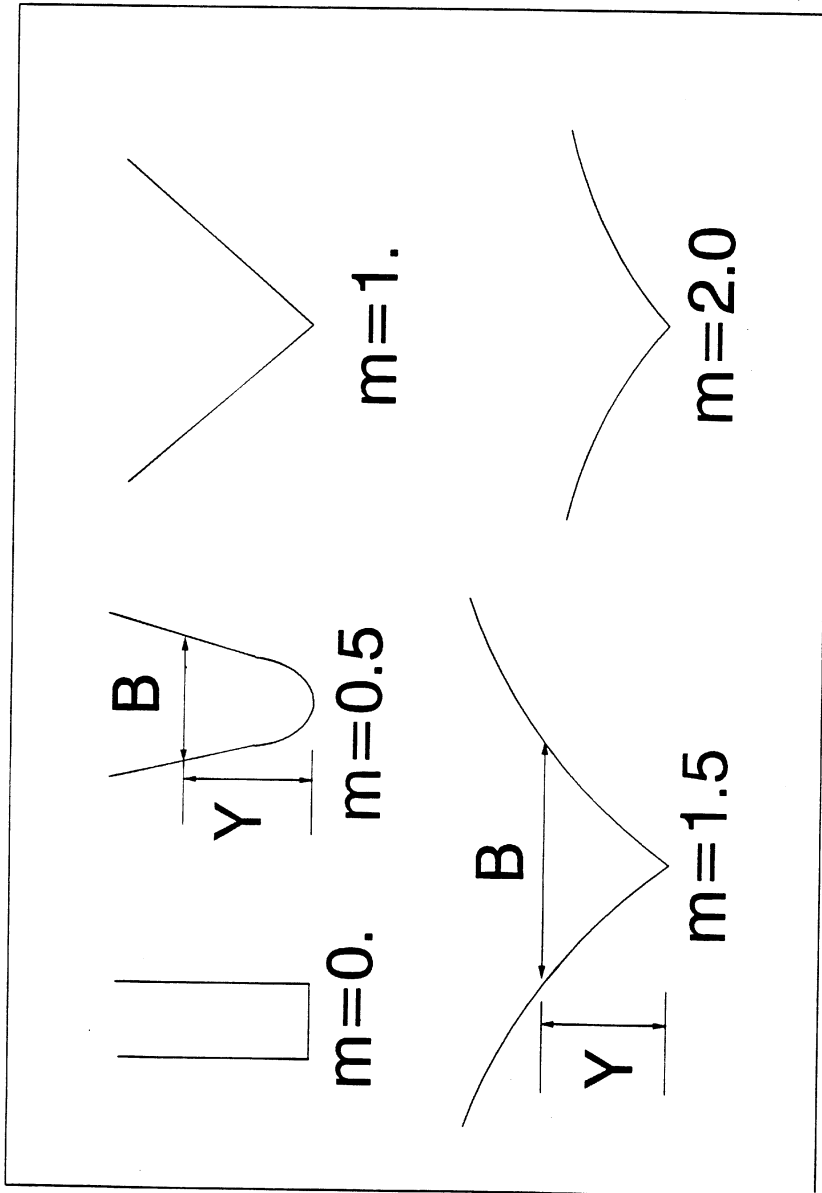


Figure 3. Sample input for Operation FLDWAV

- Column -

	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
FLDWAV																
DMOINEF																
INPUT CO																
PROBLEM DEMOINE																
Initial Conditions			4/17/98													
EOM																
NO DESC																
.01	1.0		0.6		5280.00			6.0		6		0				
2		1		10		0		0		0	1	0	0	0		
1		0		0		0										
9		1		009		1		1								
1		0		0		0		0								
1.	0.25		6.0		0			0.		0						
0	0.		0.0	- Levees on Des Moines												
21		1		21				1000.0	0.	0.	0.			Des Moines River		
32		1		32	1	19	60.	100.0	0.	0.	0.			Raccoon River		
2	3	3	3	2	-13	21	0	0	0					Des Moines River		
2	0	1	2	2	-13	33	0	0	0					Raccoon River		
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
211.40	210.60		209.50		208.60			208.00		207.00		206.40		205.50		
204.60	204.30		203.30		203.13			202.30		202.25		202.20		202.10		
201.60	201.56		201.40		201.30			200.70								
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
29.00	27.99		26.97		25.96			24.94		23.93		22.91		21.90		
20.88	19.85		18.83		17.80			16.78		15.76		14.73		13.71		
12.68	11.66		10.64		9.61			8.59		7.56		6.54		5.51		
4.49	3.45		3.04		2.54			2.04		1.82		1.01		.00		
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
	0.000		0.000		0.000			0.000		0.000		0.000		0.000		0.000
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3		GRMI4		QINE												
9		DMOI4LOC		SQIN												
20		DESI4LOC		SQIN												
25		DOSI4		QINE												
1	787.42		SDTI4G		TWEL											

Figure 3. Sample input for Operation FLDWAV

- Column -

	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
10		773.68			DMOI4C		STG									
21		762.52			DESI4G		STG									
1		841.16			VNMI4G		STG									
28		780.71			DEMI4C		STG									
1		DES21140	SSTG			0.00										
2		DES21060	SSTG			0.00										
3		DES20950	SSTG			0.00										
4		DES20860	SSTG			0.00										
5		DES20800	SSTG			0.00										
6		DES20700	SSTG			0.00										
7		DES20640	SSTG			0.00										
8		DES20550	SSTG			0.00										
9		DES20460	SSTG			0.00										
10		DES20430	SSTG			0.00										
11		DES20330	SSTG			0.00										
12		DES20313	SSTG			0.00										
13		DES20230	SSTG			0.00										
14		DES20225	SSTG			0.00										
15		DES20220	SSTG			0.00										
16		DES20210	SSTG			0.00										
17		DES20160	SSTG			0.00										
18		DES20156	SSTG			0.00										
19		DES20140	SSTG			0.00										
20		DES20130	SSTG			0.00										
21		DES20070	SSTG			0.00										
1		RAC02900	SSTG			0.00										
2		RAC02799	SSTG			0.00										
3		RAC02697	SSTG			0.00										
4		RAC02596	SSTG			0.00										
5		RAC02494	SSTG			0.00										
6		RAC02393	SSTG			0.00										
7		RAC02291	SSTG			0.00										
8		RAC02190	SSTG			0.00										

Figure 3. Sample input for Operation FLDWAV

- Column -

	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
9			RAC02088	SSTG		0.00										
10			RAC01985	SSTG		0.00										
11			RAC01883	SSTG		0.00										
12			RAC01780	SSTG		0.00										
13			RAC01678	SSTG		0.00										
14			RAC01576	SSTG		0.00										
15			RAC01473	SSTG		0.00										
16			RAC01371	SSTG		0.00										
17			RAC01268	SSTG		0.00										
18			RAC01166	SSTG		0.00										
19			RAC01064	SSTG		0.00										
20			RAC00961	SSTG		0.00										
21			RAC00859	SSTG		0.00										
22			RAC00756	SSTG		0.00										
23			RAC00654	SSTG		0.00										
24			RAC00551	SSTG		0.00										
25			RAC00449	SSTG		0.00										
26			RAC00345	SSTG		0.00										
27			RAC00304	SSTG		0.00										
28			RAC00254	SSTG		0.00										
28			DEMI4	SSTG		780.71										
29			RAC00204	SSTG		0.00										
30			RAC00182	SSTG		0.00										
31			RAC00101	SSTG		0.00										
32			RAC00000	SSTG		0.00										
100.			SDTI4	QINE												
10.			VNMI4	QINE												
DESI4																
RIVER 1	21	SECTIONS														
	0.00	0.00					1									
	784.30	789.30	792.00	792.10	799.90	800.00	805.00	810.00	820.00							
	.00	1.00	700.00	800.00	1400.00	3800.00	5000.00	8300.00	8600.00							
	.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
	0.00	0.00		2												

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
783.70	788.70	790.30	795.00	799.90	800.00	804.00	810.00	820.00							
.00	1.00	165.20	317.20	453.70	509.30	2564.70	3016.70	10500.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00		3												
781.50	786.50	790.40	791.30	799.90	800.00	803.10	810.00	820.00							
.00	1.00	220.00	315.00	500.00	1500.00	3100.00	13400.00	15000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		4										
780.50	785.50	787.70	789.30	798.60	800.00	802.00	810.00	820.00							
.00	1.00	162.30	309.40	357.80	446.00	1929.70	9500.00	10000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		5										
778.80	783.80	786.50	789.60	797.70	799.70	805.00	810.00	820.00							
.00	1.00	230.00	360.00	405.00	1260.00	7400.00	11000.00	11800.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		5										
776.70	781.70	785.60	796.20	797.70	797.90	803.50	803.70	810.00							
.00	1.00	233.20	392.80	549.70	1203.20	1260.00	3061.40	3100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		7										
776.00	781.00	784.40	784.60	790.00	795.00	800.00	804.10	810.00							
.00	1.00	150.00	220.00	500.00	1000.00	1350.00	1725.00	3100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		8										
775.20	780.20	782.10	782.30	793.40	793.60	795.50	795.60	810.00							
.00	1.00	78.60	135.40	310.40	548.10	555.90	778.50	3100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		9										
773.80	778.80	785.90	789.60	790.80	795.00	800.00	805.00	810.00							
.00	1.00	215.00	270.00	390.00	1000.00	1250.00	2700.00	3100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		10										
773.50	778.50	785.00	787.00	790.00	795.00	800.00	805.00	810.00							

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
.00	1.00	150.00	150.00	150.00	1200.00	1575.00	3000.00	4000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		11										
771.80	776.80	780.00	784.70	791.70	795.00	800.00	805.00	810.00							
.00	1.00	150.00	277.00	344.70	500.00	2300.00	2600.00	3000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		12										
770.80	775.80	781.90	783.30	791.00	795.00	800.00	805.00	810.00							
.00	1.00	150.00	270.00	345.00	400.00	800.00	1200.00	2000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		13										
768.60	773.60	775.90	777.30	780.20	790.00	795.00	800.00	810.00							
.00	1.00	90.00	240.00	330.00	350.00	375.00	1800.00	3600.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		14										
768.10	773.10	775.80	780.10	788.80	795.00	800.00	805.00	810.00							
.00	1.00	225.00	340.00	390.00	1575.00	2000.00	3100.00	4000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		15										
767.60	772.60	775.90	780.10	789.40	795.00	800.00	805.00	810.00							
.00	1.00	210.00	375.00	400.00	2750.00	3500.00	4300.00	4900.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		16										
767.00	772.00	776.40	777.80	780.00	790.00	795.00	800.00	810.00							
.00	1.00	220.00	370.00	400.00	550.00	3000.00	5100.00	6100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		17										
764.60	769.60	773.00	777.00	780.00	790.00	800.00	805.00	810.00							
.00	1.00	150.00	275.00	390.00	1100.00	4000.00	4500.00	5000.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		18										
763.00	768.00	769.70	771.10	780.00	790.00	795.00	800.00	810.00							
.00	1.00	340.00	605.00	710.00	1400.00	3500.00	6300.00	6500.00							

Figure 3. Sample input for Operation FLDWAV

- Column -

	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
	.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0.00	0.00	0.00	0.00				19								
	762.50	767.50	772.60	772.80	799.90	800.00		800.50		800.80		801.30				
	.00	1.00	280.00	520.00	610.00	760.00		800.00		825.00		870.00				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
	0.00	0.00	0.00	0.00				20								
	762.00	767.00	769.60	782.40	784.40	784.60		789.90		795.50		801.00				
	.00	1.00	290.00	390.00	460.00	630.00		670.00		1050.00		1100.00				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
	0.00	0.00	0.00	0.00				21								
	756.30	761.30	768.40	774.00	784.20	785.40		785.70		795.50		802.80				
	.00	1.00	130.00	360.00	415.00	500.00		680.00		750.00		753.00				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
RIVER 2	0.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
32 SECTIONS								1								
	836.20	841.20	843.00	845.60	850.00	855.00		860.00		865.00		870.00				
	.00	1.00	200.00	350.00	400.00	925.00		1600.00		2600.00		3400.00				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
	0.00	0.00	0.00	0.00				2								
	834.60	839.60	841.50	844.10	848.60	854.30		859.30		864.30		869.30				
	.00	1.00	210.70	350.00	400.00	846.40		2042.90		3042.90		3814.30				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
	0.00	0.00	0.00	0.00				3								
	833.00	838.00	840.00	842.60	847.10	853.60		858.60		863.60		868.60				
	.00	1.00	221.40	350.00	400.00	767.90		2485.70		3485.70		4228.60				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
	0.00	0.00	0.00	0.00				4								
	831.40	836.40	838.50	841.10	845.70	852.90		857.90		862.90		867.90				
	.00	1.00	232.10	350.00	400.00	689.30		2928.60		3928.60		4642.90				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		
	0.00	0.00	0.00	0.00				5								
	829.80	834.80	837.00	839.50	844.30	852.10		857.10		862.10		867.10				
	.00	1.00	242.90	350.00	400.00	610.70		3371.40		4371.40		5057.10				
	.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00		0.00		

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0.00	0.00	0.00	0.00				6								
828.20	833.20	835.50	838.00	842.90	851.40		856.40		861.40		866.40				
.00	1.00	253.60	350.00	400.00	532.10		3814.30		4814.30		5471.40				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				7								
826.60	831.60	834.00	836.50	841.40	850.70		855.70		860.70		865.70				
.00	1.00	264.30	350.00	400.00	453.60		4257.10		5257.10		5885.70				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				8								
825.00	830.00	832.50	835.00	840.00	850.00		855.00		860.00		865.00				
.00	1.00	275.00	350.00	400.00	475.00		4700.00		5700.00		6300.00				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
.00	.00	0.00	0.00	0.00	0.00		9								
822.20	827.20	829.70	832.20	837.10	846.50		851.80		856.80		861.80				
.00	1.00	270.60	344.10	394.10	397.10		4488.20		5617.60		6229.40				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				10								
819.50	824.50	826.90	829.40	834.10	843.00		848.50		853.50		858.50				
.00	1.00	266.20	338.20	388.20	419.10		4276.50		5535.30		6158.80				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				11								
816.70	821.70	824.10	826.50	831.20	839.40		845.30		850.30		855.30				
.00	1.00	261.80	332.40	382.40	441.20		4064.70		5452.90		6088.20				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				12								
814.00	819.00	821.30	823.70	828.20	835.90		842.10		847.10		852.10				
.00	1.00	257.40	326.50	376.50	463.20		3852.90		5370.60		6017.60				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				13								
811.20	816.20	818.50	820.90	825.30	832.40		838.80		843.80		848.80				
.00	1.00	252.90	320.60	370.60	485.30		3641.20		5288.20		5947.10				
.00	0.00	0.00	0.00	0.00	0.00		0.00		0.00		0.00				
0.00	0.00	0.00	0.00				14								

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
808.50	813.50	815.70	818.10	822.40	828.90	835.60	840.60	845.60							
.00	1.00	248.50	314.70	364.70	507.40	3429.40	5205.90	5876.50							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		15										
805.70	810.70	812.90	815.20	819.40	825.30	832.40	837.40	842.40							
.00	1.00	244.10	308.80	358.80	529.40	3217.60	5123.50	5805.90							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		16										
803.00	808.00	810.10	812.40	816.50	821.80	829.10	834.10	839.10							
.00	1.00	239.70	302.90	352.90	551.50	3005.90	5041.20	5735.30							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		17										
800.20	805.20	807.40	809.60	813.50	818.30	825.90	830.90	835.90							
.00	1.00	235.30	297.10	347.10	573.50	2794.10	4958.80	5664.70							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		18										
797.50	802.50	804.60	806.80	810.60	814.80	822.60	827.60	832.60							
.00	1.00	230.90	291.20	341.20	595.60	2582.40	4876.50	5594.10							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		19										
794.70	799.70	801.80	803.90	807.60	811.20	819.40	824.40	829.40							
.00	1.00	226.50	285.30	335.30	617.60	2370.60	4794.10	5523.50							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		20										
792.00	797.00	799.00	801.10	804.70	807.70	816.20	821.20	826.20							
.00	1.00	222.10	279.40	329.40	639.70	2158.80	4711.80	5452.90							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		21										
789.20	794.20	796.20	798.30	801.80	804.20	812.90	817.90	822.90							
.00	1.00	217.60	273.50	323.50	661.80	1947.10	4629.40	5382.40							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		22										
786.50	791.50	793.40	795.50	798.80	800.70	809.70	814.70	819.70							

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
.00	1.00	213.20	267.60	317.60	683.80	1735.30	4547.10	5311.80							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		23										
783.70	788.70	790.60	792.60	795.90	797.10	806.50	811.50	816.50							
.00	1.00	208.80	261.80	311.80	705.90	1523.50	4464.70	5241.20							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		24										
781.00	786.00	787.80	789.80	792.90	793.60	803.20	808.20	813.20							
.00	1.00	204.40	255.90	305.90	727.90	1311.80	4382.40	5170.60							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		25										
778.20	783.20	785.00	787.00	790.00	790.10	800.00	805.00	810.00							
.00	1.00	200.00	250.00	300.00	750.00	1100.00	4300.00	5100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		26										
775.10	780.10	780.80	785.00	790.00	795.00	800.00	805.00	810.00							
.00	1.00	150.00	230.00	260.00	730.00	3100.00	4700.00	6100.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00		27										
774.20	779.20	780.20	784.70	790.00	790.50	791.00	800.00	810.00							
.00	1.00	115.00	215.00	500.00	1380.00	1400.00	1800.00	2000.00							
.00	.00	.00	.00	.00	.00	540.00	1900.00	2000.00							
0.00	0.00	0.00	0.00		28										
773.00	778.00	779.20	784.70	790.00	790.50	791.00	800.00	810.00							
.00	1.00	115.00	215.00	500.00	1380.00	1400.00	1800.00	2000.00							
.00	.00	.00	.00	.00	.00	3540.00	1900.00	2000.00							
0.00	0.00	0.00	0.00		29										
772.00	777.00	779.20	784.70	790.00	790.50	791.00	800.00	810.00							
.00	1.00	115.00	215.00	500.00	1380.00	1400.00	1800.00	2000.00							
.00	.00	.00	.00	.00	.00	540.00	1900.00	2000.00							
0.00	0.00	0.00	0.00		30										
771.50	776.50	778.80	789.70	790.00	793.90	799.70	799.90	810.00							
.00	1.00	210.00	310.00	400.00	2500.00	3000.00	3825.00	7000.00							

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
.00	.00	.00	.00	70.00	800.00	500.00	.00	.00							
0.00	0.00	0.00	0.00		31										
769.50	774.50	778.00	780.00	790.00	790.10	795.00	800.00	810.00							
.00	1.00	300.00	350.00	400.00	400.00	3000.00	4300.00	5300.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0.00	0.00	0.00	0.00		32										
764.00	769.00	778.20	778.80	781.00	790.00	800.00	805.00	810.00							
.00	1.00	240.00	310.00	370.00	550.00	7000.00	8200.00	9500.00							
.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00							
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1	10														
.0140	.0150	.0170	.0210	.0220	.0250	.0300	.0480								
.0570	.0570	.0570	.0570	.0570											
0.	1250.	2500.	5000.	8000.	12000.	20000.	40000.								
60000.	80000.	120000.	140000.	160000.											
.0900	.0620	.0430	.0300	.0270	.0220	.0260	.0320								
.0320	.0400	.0400	.0500	.0500											
0.	1250.	2500.	5000.	7500.	10000.	20000.	40000.								
60000.	80000.	120000.	140000.	160000.											
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1	28														
.0480	.0290	.0310	.0330	.0410	.0410	.0450	.0450								
.0480	.0500	.0500	.0500	.0500											
0.	2500.	5000.	10000.	20000.	40000.	60000.	80000.								
120000.	130000.	140000.	150000.	160000.											
.0340	.0360	.0320	.0300	.0320	.0380	.0820	.0840								
.0900	.0800	.0890	.1100	.1100											
0.	2500.	5000.	7500.	10000.	15000.	20000.	25000.								
30000.	35000.	40000.	45000.	65000											
INITIAL WATER SURFACE ELEVATIONS FOR RIVER NO. 1															
796.90	796.48	794.79	793.43	792.93	792.22	791.90	791.09								
789.88	788.58	785.57	784.62	781.39	781.40	781.35	781.26								
780.76	780.84	780.81	780.63	780.35											
INITIAL WATER SURFACE ELEVATIONS FOR RIVER NO. 2															
850.37	848.69	847.04	845.41	843.74	842.04	840.20	837.90								
835.19	832.44	829.72	826.98	824.26	821.54	818.81	816.13								
813.49	810.93	808.43	805.92	803.38	800.62	797.71	794.56								
792.14	790.05	789.32	788.01	785.56	784.67	782.95	780.72								
INITIAL DISCHARGES FOR RIVER NO. 1															
6292.46	6289.99	6288.39	6789.13	6789.02	6789.20	6789.65	6790.33								
6790.87	6823.60	6824.38	6825.23	6819.51	6818.31	6816.85	6812.62								
6817.68	6821.54	6844.42	13184.00	13264.52											
INITIAL DISCHARGES FOR RIVER NO. 2															
7617.82	7576.88	7544.37	7515.11	7486.06	7458.10	7431.10	7405.62								
7380.34	7354.39	7328.24	7301.32	7273.92	7245.68	7216.01	7184.46								
7145.55	7095.82	7036.38	6969.46	6897.56	6824.21	6757.67	6695.50								

Figure 3. Sample input for Operation FLDWAV

- Column -

5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
6607.23	6591.06	6570.89	6546.37	6528.81	6527.50	6487.31	6484.10								
INITIAL LATERAL INFLOWS FOR RIVER NO. 1															
501.56	32.61	47.01													
INITIAL LATERAL INFLOWS FOR RIVER NO. 2															
62.93															

Figure 4. Sample output from Operation FLDWAV parameter print routine

```

*****
FLDWAV OPERATION NAME=DMOINEF PREVIOUS NAME=INPUT CO
*****

PROGRAM FLDWAV - VERSION 1.0 9/30/98

HYDROLOGIC RESEARCH LABORATORY
W/OH1 OFFICE OF HYDROLOGY
NOAA, NATIONAL WEATHER SERVICE
SILVER SPRING, MARYLAND 20910

*****
*****
*** SUMMARY OF INPUT DATA ***
*****
*****

PROBLEM DEMOINE

      EPSY THETA F1 XFACT DTHYD DTOUT METRIC
      .010 1.000 .600 5280.000 6.000 6.000 0

      JN NU ITMAX KWARM KFLP NET ICOND FUTURE DATA
      2 1 10 0 0 0 1 0 0 0

      NYQD KCG NCG KPRES
      112 0 0 0

      NCS KPL JNK KREVRS NFGRF
      9 1 9 1 1

      IOBS KTERM NP NPST NPEND
      1 0 0 0 0

      TEH DTHII DTHPLT FRDFR DTEXP MDT
      1.000 .25000 6.00000 .00 .00000 20

      NLEV DHLV DTHLV
      0 .00000 .00000

RIVER NO. NBT NPT1 NPT2 MRV NJUN ATF EPQJ COFW VWIND WINAGL
      1 21 1 21 1 19 60.00 1000.00 .00 .00 .00
      2 32 1 32 1 19 60.00 100.00 .00 .00 .00

RIVER NO. KU KD NQL NGAGE NRCM1 NQCM NSTR FUTURE DATA
      1 2 3 3 3 2 -13 21 0 0 0
      2 2 0 1 2 2 -13 33 0 0 0

RIVER NO. MIXF MUD KFTR KLOS FUTURE DATA
      1 0 0 0 0 0 0 0 0 0 0
      2 0 0 0 0 0 0 0 0 0 0

XT(I, 1) I=1,NB( 1)
      211.400 210.600 209.500 208.600 208.000 207.000 206.400 205.500
      204.600 204.300 203.300 203.130 202.300 202.250 202.200 202.100
      201.600 201.560 201.400 201.300 200.700

DXM(I, 1) I=1,NB( 1)
      .000 .000 .000 .000 .000 .000 .000 .000
      .000 .000 .000 .000 .000 .000 .000 .000
      .000 .000 .000 .000

KRCHT(I, 1) I=1,NRCH
      0 0 0 0 0 0
      0 0 0 0 0 0
      0 0 0 0

XT(I, 2) I=1,NB( 2)
      29.000 27.990 26.970 25.960 24.940 23.930 22.910 21.900
      20.880 19.850 18.830 17.800 16.780 15.760 14.730 13.710
      12.680 11.660 10.640 9.610 8.590 7.560 6.540 5.510

```

Figure 4. Sample output from Operation FLDWAV parameter print routine

	4.490	3.450	3.040	2.540	2.040	1.820	1.010	.000
DXM(I, 2) I=1,NB(2)	.000	.000	.000	.000	.000	.000	.000	.000
	.000	.000	.000	.000	.000	.000	.000	.000
	.000	.000	.000	.000	.000	.000	.000	.000
	.000	.000	.000	.000	.000	.000	.000	.000

KRCHT(I, 2) I=1,NRCH	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0

LOCAL (LATERAL) FLOW INFO FOR RIVER J= 1

L-FLOW (I)	LQ1 (I, J)	ID	TYPE
1	3	GRMI4	QINE
2	9	DMOI4LOC	SQIN
3	20	DESI4LOC	SQIN

LOCAL (LATERAL) FLOW INFO FOR RIVER J= 2

L-FLOW (I)	LQ1 (I, J)	ID	TYPE
1	25	DOSI4	QINE

PLOTTING T.S.INFO FOR RIVER J= 1

STATION (I)	NGS (I, J)	GZ (I, J)	ID	TYPE
1	1	787.42	SDTI4G	TWEL
2	10	773.68	DMOI4C	STG
3	21	762.52	DESI4G	STG

PLOTTING T.S.INFO FOR RIVER J= 2

STATION (I)	NGS (I, J)	GZ (I, J)	ID	TYPE
1	1	841.16	VNMI4G	STG
2	28	780.71	DEMI4C	STG

OUTPUT T.S. INFO FOR RIVER J= 1

STATION (I)	NST (I, J)	ID	TYPE	GZO (I, J)
1	1	DES21140	SSTG	.00
2	2	DES21060	SSTG	.00
3	3	DES20950	SSTG	.00
4	4	DES20860	SSTG	.00
5	5	DES20800	SSTG	.00
6	6	DES20700	SSTG	.00
7	7	DES20640	SSTG	.00
8	8	DES20550	SSTG	.00
9	9	DES20460	SSTG	.00
10	10	DES20430	SSTG	.00
11	11	DES20330	SSTG	.00
12	12	DES20313	SSTG	.00
13	13	DES20230	SSTG	.00
14	14	DES20225	SSTG	.00
15	15	DES20220	SSTG	.00
16	16	DES20210	SSTG	.00
17	17	DES20160	SSTG	.00
18	18	DES20156	SSTG	.00
19	19	DES20140	SSTG	.00
20	20	DES20130	SSTG	.00
21	21	DES20070	SSTG	.00

OUTPUT T.S. INFO FOR RIVER J= 2

STATION (I)	NST (I, J)	ID	TYPE	GZO (I, J)
1	1	RAC02900	SSTG	.00
2	2	RAC02799	SSTG	.00
3	3	RAC02697	SSTG	.00
4	4	RAC02596	SSTG	.00
5	5	RAC02494	SSTG	.00
6	6	RAC02393	SSTG	.00
7	7	RAC02291	SSTG	.00
8	8	RAC02190	SSTG	.00
9	9	RAC02088	SSTG	.00
10	10	RAC01985	SSTG	.00
11	11	RAC01883	SSTG	.00

Figure 4. Sample output from Operation FLDWAV parameter print routine

12	12	RAC01780	SSTG	.00
13	13	RAC01678	SSTG	.00
14	14	RAC01576	SSTG	.00
15	15	RAC01473	SSTG	.00
16	16	RAC01371	SSTG	.00
17	17	RAC01268	SSTG	.00
18	18	RAC01166	SSTG	.00
19	19	RAC01064	SSTG	.00
20	20	RAC00961	SSTG	.00
21	21	RAC00859	SSTG	.00
22	22	RAC00756	SSTG	.00
23	23	RAC00654	SSTG	.00
24	24	RAC00551	SSTG	.00
25	25	RAC00449	SSTG	.00
26	26	RAC00345	SSTG	.00
27	27	RAC00304	SSTG	.00
28	28	RAC00254	SSTG	.00
29	28	DEMI4	SSTG	780.71
30	29	RAC00204	SSTG	.00
31	30	RAC00182	SSTG	.00
32	31	RAC00101	SSTG	.00
33	32	RAC00000	SSTG	.00

UPSTREAM BOUNDARY INFORMATION

RIVER NO	MIN Q/H	GAGE ZERO	ID	TYPE
1	100.00		SDTI4	QINE
2	10.00		VNMI4	QINE

DOWSTREAM BOUNDARY INFORMATION

RATING CURVE ID:DESI4

RIVER NO. 1

I=	1	FLDSTG=	.00	X=	211.40	KRCH=	0.	FKEC=	.00
		HS=	784.30		789.30		792.00		799.90
810.00		820.00							800.00
		BS=	.0		1.0		700.0		800.0
8300.0		8600.0							1400.0
		AS=	.0		2.5		948.9		1023.8
65113.8		149613.8							9603.9
		BSS=	.0		.0		.0		.0
.0		.0			.0		.0		.0
.0		ASS=	.0		.0		.0		.0
		.0			.0		.0		.0

I=	2	FLDSTG=	.00	X=	210.60	KRCH=	0.	FKEC=	.00
		HS=	783.70		788.70		790.30		799.90
810.00		820.00							800.00
		BS=	.0		1.0		165.2		317.2
3016.7		10500.0							453.7
		AS=	.0		2.5		135.5		1269.1
26098.2		93681.7							3157.8
		BSS=	.0		.0		.0		.0
.0		.0			.0		.0		.0
.0		ASS=	.0		.0		.0		.0
		.0			.0		.0		.0

I=	3	FLDSTG=	.00	X=	209.50	KRCH=	0.	FKEC=	.00
		HS=	781.50		786.50		790.40		799.90
810.00		820.00							800.00
		BS=	.0		1.0		220.0		315.0
13400.0		15000.0							500.0
		AS=	.0		2.5		433.5		674.2
68333.8		210333.8							4178.7
		BSS=	.0		.0		.0		.0
.0		.0			.0		.0		.0
.0		ASS=	.0		.0		.0		.0
		.0			.0		.0		.0

I=	4	FLDSTG=	.00	X=	208.60	KRCH=	0.	FKEC=	.00
		HS=	780.50		785.50		787.70		798.60
810.00		820.00							800.00
		BS=	.0		1.0		162.3		309.4
9500.0		10000.0							357.8
		AS=	.0		2.5		182.1		559.5
									3662.0
									4224.6
									6600.3

Figure 4. Sample output from Operation FLDWAV parameter print routine

```

52319.1 149819.1
.0      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=      5      FLDSTG= .00      X=      208.00      KRCH=      0.      FKEC=      .00
810.00      HS=      778.80      783.80      786.50      789.60      797.70      799.70      805.00
11000.0      BS=      .0      1.0      230.0      360.0      405.0      1260.0      7400.0
74941.1      AS=      .0      2.5      314.4      1228.8      4327.1      5992.1      28941.1
188941.1      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=      6      FLDSTG= .00      X=      207.00      KRCH=      0.      FKEC=      .00
803.70      HS=      776.70      781.70      785.60      796.20      797.70      797.90      803.50
3061.4      BS=      .0      1.0      233.2      392.8      549.7      1203.2      1260.0
11988.3      AS=      .0      2.5      459.2      3777.0      4483.9      4659.2      11556.1
.0      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=      7      FLDSTG= .00      X=      206.40      KRCH=      0.      FKEC=      .00
804.10      HS=      776.00      781.00      784.40      784.60      790.00      795.00      800.00
1725.0      BS=      .0      1.0      150.0      220.0      500.0      1000.0      1350.0
18168.9      AS=      .0      2.5      259.2      296.2      2240.2      5990.2      11865.2
32402.7      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=      8      FLDSTG= .00      X=      205.50      KRCH=      0.      FKEC=      .00
795.60      HS=      775.20      780.20      782.10      782.30      793.40      793.60      795.50
778.5      BS=      .0      1.0      78.6      135.4      310.4      548.1      555.9
3775.1      AS=      .0      2.5      78.1      99.5      2573.7      2659.5      3708.4
31700.3      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=      9      FLDSTG= .00      X=      204.60      KRCH=      0.      FKEC=      .00
805.00      HS=      773.80      778.80      785.90      789.60      790.80      795.00      800.00
2700.0      BS=      .0      1.0      215.0      270.0      390.0      1000.0      1250.0
20481.6      AS=      .0      2.5      769.3      1666.5      2062.5      4981.6      10606.6
34981.6      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=      10     FLDSTG= .00      X=      204.30      KRCH=      0.      FKEC=      .00
805.00      HS=      773.50      778.50      785.00      787.00      790.00      795.00      800.00
3000.0      BS=      .0      1.0      150.0      150.0      150.0      1200.0      1575.0
22993.3      AS=      .0      2.5      493.3      793.3      1243.3      4618.3      11555.8
40493.3      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

```


Figure 4. Sample output from Operation FLDWAV parameter print routine

I=	11	FLDSTG=	.00	X=	203.30	KRCH=	0.	FKEC=	.00
805.00	HS=	771.80	776.80	780.00	784.70	791.70	795.00	800.00	
	810.00								
2600.0	BS=	.0	1.0	150.0	277.0	344.7	500.0	2300.0	
	3000.0								
24067.3	AS=	.0	2.5	244.1	1247.6	3423.5	4817.3	11817.3	
	38067.3								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	12	FLDSTG=	.00	X=	203.13	KRCH=	0.	FKEC=	.00
805.00	HS=	770.80	775.80	781.90	783.30	791.00	795.00	800.00	
	810.00								
1200.0	BS=	.0	1.0	150.0	270.0	345.0	400.0	800.0	
	2000.0								
12614.8	AS=	.0	2.5	463.1	757.0	3124.8	4614.8	7614.8	
	20614.8								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	13	FLDSTG=	.00	X=	202.30	KRCH=	0.	FKEC=	.00
800.00	HS=	768.60	773.60	775.90	777.30	780.20	790.00	795.00	
	810.00								
1800.0	BS=	.0	1.0	90.0	240.0	330.0	350.0	375.0	
	3600.0								
11746.6	AS=	.0	2.5	107.2	338.1	1164.7	4496.6	6309.1	
	38746.6								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	14	FLDSTG=	.00	X=	202.25	KRCH=	0.	FKEC=	.00
805.00	HS=	768.10	773.10	775.80	780.10	788.80	795.00	800.00	
	810.00								
3100.0	BS=	.0	1.0	225.0	340.0	390.0	1575.0	2000.0	
	4000.0								
32476.9	AS=	.0	2.5	307.6	1522.3	4697.9	10789.4	19726.9	
	50226.9								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	15	FLDSTG=	.00	X=	202.20	KRCH=	0.	FKEC=	.00
805.00	HS=	767.60	772.60	775.90	780.10	789.40	795.00	800.00	
	810.00								
4300.0	BS=	.0	1.0	210.0	375.0	400.0	2750.0	3500.0	
	4900.0								
49127.9	AS=	.0	2.5	350.7	1579.1	5182.9	14002.9	29627.9	
	72127.9								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	16	FLDSTG=	.00	X=	202.10	KRCH=	0.	FKEC=	.00
800.00	HS=	767.00	772.00	776.40	777.80	780.00	790.00	795.00	
	810.00								
5100.0	BS=	.0	1.0	220.0	370.0	400.0	550.0	3000.0	
	6100.0								
35623.7	AS=	.0	2.5	488.7	901.7	1748.7	6498.7	15373.7	
	91623.7								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	17	FLDSTG=	.00	X=	201.60	KRCH=	0.	FKEC=	.00
805.00	HS=	764.60	769.60	773.00	777.00	780.00	790.00	800.00	
	810.00								
4500.0	BS=	.0	1.0	150.0	275.0	390.0	1100.0	4000.0	
	5000.0								
	AS=	.0	2.5	259.2	1109.2	2106.7	9556.7	35056.7	

Figure 4. Sample output from Operation FLDWAV parameter print routine

```

56306.7  80056.7
.0      BSS=      .0      .0      .0      .0      .0      .0      .0
.0      ASS=      .0      .0      .0      .0      .0      .0      .0
.0      .0

I=  18  FLDSTG=  .00  X=  201.56  KRCH=  0.  FKEC=  .00
800.00  HS=  763.00  768.00  769.70  771.10  780.00  790.00  795.00
6300.0  BS=  .0      1.0      340.0   605.0   710.0   1400.0  3500.0
54105.6 AS=  .0      2.5      292.4   953.8   6805.6  17355.6 29605.6
.0      BSS=  .0      .0      .0      .0      .0      .0      .0
.0      ASS=  .0      .0      .0      .0      .0      .0      .0
.0      .0

I=  19  FLDSTG=  .00  X=  201.40  KRCH=  0.  FKEC=  .00
800.80  HS=  762.50  767.50  772.60  772.80  799.90  800.00  800.50
825.0   BS=  .0      1.0      280.0   520.0   610.0   760.0   800.0
16812.8 AS=  .0      2.5      719.0   799.1   16110.6 16179.1 16569.1
.0      BSS=  .0      .0      .0      .0      .0      .0      .0
.0      ASS=  .0      .0      .0      .0      .0      .0      .0
.0      .0

I=  20  FLDSTG=  .00  X=  201.30  KRCH=  0.  FKEC=  .00
795.50  HS=  762.00  767.00  769.60  782.40  784.40  784.60  789.90
1050.0  BS=  .0      1.0      290.0   390.0   460.0   630.0   670.0
13952.8 AS=  .0      2.5      380.8   4732.8  5582.8  5691.8  9136.8
.0      BSS=  .0      .0      .0      .0      .0      .0      .0
.0      ASS=  .0      .0      .0      .0      .0      .0      .0
.0      .0

I=  21  FLDSTG=  .00  X=  200.70
795.50  HS=  756.30  761.30  768.40  774.00  784.20  785.40  785.70
750.0   BS=  .0      1.0      130.0   360.0   415.0   500.0   680.0
13525.0 AS=  .0      2.5      467.6   1839.5  5792.1  6341.1  6518.0
.0      BSS=  .0      .0      .0      .0      .0      .0      .0
.0      ASS=  .0      .0      .0      .0      .0      .0      .0
.0      .0

RIVER NO.  2

I=  1   FLDSTG=  .00  X=  29.00  KRCH=  0.  FKEC=  .00
865.00  HS=  836.20  841.20  843.00  845.60  850.00  855.00  860.00
2600.0  BS=  .0      1.0      200.0   350.0   400.0   925.0   1600.0
22673.4 AS=  .0      2.5      183.4   898.4   2548.4  5860.9  12173.4
.0      BSS=  .0      .0      .0      .0      .0      .0      .0
.0      ASS=  .0      .0      .0      .0      .0      .0      .0
.0      .0

I=  2   FLDSTG=  .00  X=  27.99  KRCH=  0.  FKEC=  .00
864.30  HS=  834.60  839.60  841.50  844.10  848.60  854.30  859.30
3042.9  BS=  .0      1.0      210.7   350.0   400.0   846.4   2042.9
26110.0 AS=  .0      2.5      203.6   932.5   2620.0  6172.3  13395.5
.0      BSS=  .0      .0      .0      .0      .0      .0      .0
.0      ASS=  .0      .0      .0      .0      .0      .0      .0
.0      .0

```

Figure 4. Sample output from Operation FLDWAV parameter print routine

.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	3	FLDSTG=	.00	X=	26.97	KRCH=	0.	FKEC=	.00
863.60	HS=	833.00	838.00	840.00	842.60	847.10	853.60	858.60	
	868.60								
3485.7	BS=	.0	1.0	221.4	350.0	400.0	767.9	2485.7	
	4228.6								
29513.4	AS=	.0	2.5	224.9	967.7	2655.2	6450.9	14584.9	
	48799.1								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	4	FLDSTG=	.00	X=	25.96	KRCH=	0.	FKEC=	.00
862.90	HS=	831.40	836.40	838.50	841.10	845.70	852.90	857.90	
	867.90								
3928.6	BS=	.0	1.0	232.1	350.0	400.0	689.3	2928.6	
	4642.9								
32838.2	AS=	.0	2.5	247.3	1004.0	2729.0	6650.5	15695.2	
	54267.0								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	5	FLDSTG=	.00	X=	24.94	KRCH=	0.	FKEC=	.00
862.10	HS=	829.80	834.80	837.00	839.50	844.30	852.10	857.10	
	867.10								
4371.4	BS=	.0	1.0	242.9	350.0	400.0	610.7	3371.4	
	5057.1								
36065.9	AS=	.0	2.5	270.8	1011.9	2811.9	6753.6	16708.9	
	59637.1								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	6	FLDSTG=	.00	X=	23.93	KRCH=	0.	FKEC=	.00
861.40	HS=	828.20	833.20	835.50	838.00	842.90	851.40	856.40	
	866.40								
4814.3	BS=	.0	1.0	253.6	350.0	400.0	532.1	3814.3	
	5471.4								
39286.2	AS=	.0	2.5	295.3	1049.8	2887.3	6848.7	17714.7	
	65000.5								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	7	FLDSTG=	.00	X=	22.91	KRCH=	0.	FKEC=	.00
860.70	HS=	826.60	831.60	834.00	836.50	841.40	850.70	855.70	
	865.70								
5257.1	BS=	.0	1.0	264.3	350.0	400.0	453.6	4257.1	
	5885.7								
42457.7	AS=	.0	2.5	320.9	1088.7	2926.2	6895.5	18672.2	
	70314.7								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	8	FLDSTG=	.00	X=	21.90	KRCH=	0.	FKEC=	.00
860.00	HS=	825.00	830.00	832.50	835.00	840.00	850.00	855.00	
	865.00								
5700.0	BS=	.0	1.0	275.0	350.0	400.0	475.0	4700.0	
	6300.0								
46316.3	AS=	.0	2.5	347.5	1128.8	3003.8	7378.8	20316.3	
	76316.3								
.0	BSS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
.0	ASS=	.0	.0	.0	.0	.0	.0	.0	
	.0								
I=	9	FLDSTG=	.00	X=	20.88	KRCH=	0.	FKEC=	.00
856.80	HS=	822.20	827.20	829.70	832.20	837.10	846.50	851.80	
	861.80								

Figure 4. Sample output from Operation FLDWAV parameter print routine

5617.6	BS=	.0	1.0	270.6	344.1	394.1	397.1	4488.2
	6229.4							
44848.1	AS=	.0	2.5	342.0	1110.4	2919.0	6637.6	19583.6
	74465.6							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 10	FLDSTG=	.00	X= 19.85	KRCH=	0.	FKEC=	.00	
	HS=	819.50	824.50	826.90	829.40	834.10	843.00	848.50
853.50	858.50							
	BS=	.0	1.0	266.2	338.2	388.2	419.1	4276.5
5535.3	6158.8							
	AS=	.0	2.5	323.1	1078.6	2785.7	6378.2	19291.1
43820.6	73055.8							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 11	FLDSTG=	.00	X= 18.83	KRCH=	0.	FKEC=	.00	
	HS=	816.70	821.70	824.10	826.50	831.20	839.40	845.30
850.30	855.30							
	BS=	.0	1.0	261.8	332.4	382.4	441.2	4064.7
5452.9	6088.2							
	AS=	.0	2.5	317.9	1030.9	2710.7	6087.5	19379.8
43173.8	72026.5							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 12	FLDSTG=	.00	X= 17.80	KRCH=	0.	FKEC=	.00	
	HS=	814.00	819.00	821.30	823.70	828.20	835.90	842.10
847.10	852.10							
	BS=	.0	1.0	257.4	326.5	376.5	463.2	3852.9
5370.6	6017.6							
	AS=	.0	2.5	299.7	1000.3	2582.1	5814.9	19194.8
42253.5	70724.0							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 13	FLDSTG=	.00	X= 16.78	KRCH=	0.	FKEC=	.00	
	HS=	811.20	816.20	818.50	820.90	825.30	832.40	838.80
843.80	848.80							
	BS=	.0	1.0	252.9	320.6	370.6	485.3	3641.2
5288.2	5947.1							
	AS=	.0	2.5	294.5	982.7	2503.3	5541.8	18746.5
41070.0	69158.3							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 14	FLDSTG=	.00	X= 15.76	KRCH=	0.	FKEC=	.00	
	HS=	808.50	813.50	815.70	818.10	822.40	828.90	835.60
840.60	845.60							
	BS=	.0	1.0	248.5	314.7	364.7	507.4	3429.4
5205.9	5876.5							
	AS=	.0	2.5	277.0	952.8	2413.5	5247.8	18436.0
40024.3	67730.3							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 15	FLDSTG=	.00	X= 14.73	KRCH=	0.	FKEC=	.00	
	HS=	805.70	810.70	812.90	815.20	819.40	825.30	832.40
837.40	842.40							
	BS=	.0	1.0	244.1	308.8	358.8	529.4	3217.6
5123.5	5805.9							
	AS=	.0	2.5	272.1	907.9	2309.9	4930.1	18232.0
39084.8	66408.3							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							

Figure 4. Sample output from Operation FLDWAV parameter print routine

```

.0      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 16   FLDSTG=    .00   X= 13.71   KRCH=    0.   FKEC=    .00
834.10  HS= 803.00   808.00   810.10   812.40   816.50   821.80   829.10
      BS=          .0      1.0      239.7    302.9    352.9    551.5    3005.9
5041.2  AS= 5735.3   .0      2.5      255.2    879.2    2223.6   4620.3   17604.8
37722.5 64663.8     .0      .0      .0      .0      .0      .0      .0
      BSS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 17   FLDSTG=    .00   X= 12.68   KRCH=    0.   FKEC=    .00
830.90  HS= 800.20   805.20   807.40   809.60   813.50   818.30   825.90
      BS=          .0      1.0      235.3    297.1    347.1    573.5    2794.1
4958.8  AS= 5664.7   .0      2.5      262.4    848.1    2104.3   4313.7   17110.6
36492.9 63051.6     .0      .0      .0      .0      .0      .0      .0
      BSS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 18   FLDSTG=    .00   X= 11.66   KRCH=    0.   FKEC=    .00
827.60  HS= 797.50   802.50   804.60   806.80   810.60   814.80   822.60
      BS=          .0      1.0      230.9    291.2    341.2    595.6    2582.4
4876.5  AS= 5594.1   .0      2.5      246.0    820.3    2021.9   3989.1   16383.3
35030.6 61207.1     .0      .0      .0      .0      .0      .0      .0
      BSS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 19   FLDSTG=    .00   X= 10.64   KRCH=    0.   FKEC=    .00
824.40  HS= 794.70   799.70   801.80   803.90   807.60   811.20   819.40
      BS=          .0      1.0      226.5    285.3    335.3    617.6    2370.6
4794.1  AS= 5523.5   .0      2.5      241.4    778.8    1926.9   3642.1   15893.7
33805.5 59599.5     .0      .0      .0      .0      .0      .0      .0
      BSS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 20   FLDSTG=    .00   X= 9.61    KRCH=    0.   FKEC=    .00
821.20  HS= 792.00   797.00   799.00   801.10   804.70   807.70   816.20
      BS=          .0      1.0      222.1    279.4    329.4    639.7    2158.8
4711.8  AS= 5452.9   .0      2.5      225.6    752.2    1848.0   3301.7   15195.3
32371.8 57783.5     .0      .0      .0      .0      .0      .0      .0
      BSS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 21   FLDSTG=    .00   X= 8.59    KRCH=    0.   FKEC=    .00
817.90  HS= 789.20   794.20   796.20   798.30   801.80   804.20   812.90
      BS=          .0      1.0      217.6    273.5    323.5    661.8    1947.1
4629.4  AS= 5382.4   .0      2.5      221.1    736.7    1781.5   2963.9   14312.6
30753.9 55783.4     .0      .0      .0      .0      .0      .0      .0
      BSS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

I= 22   FLDSTG=    .00   X= 7.56    KRCH=    0.   FKEC=    .00
814.70  HS= 786.50   791.50   793.40   795.50   798.80   800.70   809.70
      BS=          .0      .0      .0      .0      .0      .0      .0
      .0
      ASS=          .0      .0      .0      .0      .0      .0      .0
      .0

```

Figure 4. Sample output from Operation FLDWAV parameter print routine

4547.1	BS=	.0	1.0	213.2	267.6	317.6	683.8	1735.3
	5311.8							
29219.7	AS=	.0	2.5	206.0	710.8	1676.4	2627.7	13513.7
	53866.9							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 23	FLDSTG=	.00	X=	6.54	KRCH=	0.	FKEC=	.00
	HS=	783.70		788.70	790.60	792.60	795.90	797.10
811.50	816.50							806.50
4464.7	BS=	.0	1.0	208.8	261.8	311.8	705.9	1523.5
	5241.2							
27678.2	AS=	.0	2.5	201.8	672.4	1618.9	2229.5	12707.7
	51942.9							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 24	FLDSTG=	.00	X=	5.51	KRCH=	0.	FKEC=	.00
	HS=	781.00		786.00	787.80	789.80	792.90	793.60
808.20	813.20							803.20
4382.4	BS=	.0	1.0	204.4	255.9	305.9	727.9	1311.8
	5170.6							
25906.4	AS=	.0	2.5	187.4	647.7	1518.5	1880.3	11670.9
	49788.9							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 25	FLDSTG=	.00	X=	4.49	KRCH=	0.	FKEC=	.00
	HS=	778.20		783.20	785.00	787.00	790.00	790.10
805.00	810.00							800.00
4300.0	BS=	.0	1.0	200.0	250.0	300.0	750.0	1100.0
	5100.0							
24168.4	AS=	.0	2.5	183.4	633.4	1458.4	1510.9	10668.4
	47668.4							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 26	FLDSTG=	.00	X=	3.45	KRCH=	0.	FKEC=	.00
	HS=	775.10		780.10	780.80	785.00	790.00	795.00
805.00	810.00							800.00
4700.0	BS=	.0	1.0	150.0	230.0	260.0	730.0	3100.0
	6100.0							
33628.4	AS=	.0	2.5	55.4	853.4	2078.4	4553.4	14128.4
	60628.4							
.0	BSS=	.0	.0	.0	.0	.0	.0	.0
	.0							
.0	ASS=	.0	.0	.0	.0	.0	.0	.0
	.0							
I= 27	FLDSTG=	.00	X=	3.04	KRCH=	0.	FKEC=	.00
	HS=	774.20		779.20	780.20	784.70	790.00	790.50
800.00	810.00							791.00
1800.0	BS=	.0	1.0	115.0	215.0	500.0	1380.0	1400.0
	2000.0							
18262.7	AS=	.0	2.5	60.5	803.0	2697.7	3167.7	3862.7
	37262.7							
1900.0	BSS=	.0	.0	.0	.0	.0	.0	540.0
	2000.0							
11115.0	ASS=	.0	.0	.0	.0	.0	.0	135.0
	30615.0							
I= 28	FLDSTG=	.00	X=	2.54	KRCH=	0.	FKEC=	.00
	HS=	773.00		778.00	779.20	784.70	790.00	790.50
800.00	810.00							791.00
1800.0	BS=	.0	1.0	115.0	215.0	500.0	1380.0	1400.0
	2000.0							
18439.3	AS=	.0	2.5	72.1	979.6	2874.3	3344.3	4039.3
	37439.3							
1900.0	BSS=	.0	.0	.0	.0	.0	.0	3540.0
	2000.0							

Figure 4. Sample output from Operation FLDWAV parameter print routine

```

25365.0  ASS=          .0      .0      .0      .0      .0      .0      885.0
          44865.0

  I=  29  FLDSTG=   .00    X=    2.04  KRCH=    0.  FKEC=   .00
800.00   HS=    772.00   777.00   779.20   784.70   790.00   790.50   791.00
          810.00
1800.0   BS=          .0      1.0     115.0    215.0    500.0    1380.0   1400.0
          2000.0
18497.3  AS=          .0      2.5     130.1    1037.6   2932.3   3402.3   4097.3
          37497.3
1900.0   BSS=         .0      .0      .0      .0      .0      .0      540.0
          2000.0
11115.0  ASS=          .0      .0      .0      .0      .0      .0      135.0
          30615.0

  I=  30  FLDSTG=   .00    X=    1.82  KRCH=    0.  FKEC=   .00
799.90   HS=    771.50   776.50   778.80   789.70   790.00   793.90   799.70
          810.00
3825.0   BS=          .0      1.0     210.0    310.0    400.0    2500.0   3000.0
          7000.0
25473.2  AS=          .0      2.5     245.1    3079.2   3185.7   8840.7   24790.7
          80139.3
.0       BSS=         .0      .0      .0      .0      70.0     800.0    500.0
          .0
5527.0  ASS=          .0      .0      .0      .0      10.5     1707.0   5477.0
          5527.0

  I=  31  FLDSTG=   .00    X=    1.01  KRCH=    0.  FKEC=   .00
800.00   HS=    769.50   774.50   778.00   780.00   790.00   790.10   795.00
          810.00
4300.0   BS=          .0      1.0     300.0    350.0    400.0    400.0    3000.0
          5300.0
31549.3  AS=          .0      2.5     529.3    1179.3   4929.3   4969.2   13299.3
          79549.3
.0       BSS=         .0      .0      .0      .0      .0      .0      .0
          .0
.0       ASS=         .0      .0      .0      .0      .0      .0      .0
          .0

  I=  32  FLDSTG=   .00    X=     .00
805.00   HS=    764.00   769.00   778.20   778.80   781.00   790.00   800.00
          810.00
8200.0   BS=          .0      1.0     240.0    310.0    370.0    550.0    7000.0
          9500.0
81914.1  AS=          .0      2.5    1111.1   1276.1   2024.1   6164.1   43914.1
          126164.1
.0       BSS=         .0      .0      .0      .0      .0      .0      .0
          .0
.0       ASS=         .0      .0      .0      .0      .0      .0      .0
          .0

```

REACH INFO RIVER NO. 1

```

      NCM(K, 1), K=1,NRCM1( 1)
      1  10
      CM(K, 1, 1)= .0140 .0150 .0170 .0210 .0220 .0250 .0300
.0480 .0570 .0570
      YQCM(K, 1, 1)= .0570 .0570 .0570
40000. 60000. 80000. 120000. 140000. 160000.
      CM(K, 2, 1)= .0900 .0620 .0430 .0300 .0270 .0220 .0260
.0320 .0320 .0400
      YQCM(K, 2, 1)= .0400 .0500 .0500
40000. 60000. 80000. 120000. 140000. 160000.

```

REACH INFO RIVER NO. 2

```

      NCM(K, 2), K=1,NRCM1( 2)
      1  28
      CM(K, 1, 2)= .0480 .0290 .0310 .0330 .0410 .0410 .0450

```

Figure 4. Sample output from Operation FLDWAV parameter print routine

```
.0450      .0480      .0500
          YQCM(K, 1, 2)=      .0500      .0500      .0500
80000.    120000.    130000.      0.      2500.      5000.      10000.      20000.      40000.      60000.
          140000.    150000.    160000.

          CM(K, 2, 2)=      .0340      .0360      .0320      .0300      .0320      .0380      .0820
.0840      .0900      .0800
          YQCM(K, 2, 2)=      .0890      .1100      .1100
25000.    30000.    35000.      0.      2500.      5000.      7500.      10000.      15000.      20000.
          40000.      45000.      65000.
```


Figure 5. Bottom slope profile

RIVER NO	SECT NO	X MILE	BED ELEV. FEET	REACH NO	LENGTH MILE	SLOPE FPM	ROUTING	STRUCT.
1	1	.00	5220.00	1	16.00	12.06	IMP(SUB)	
1	2	16.00	5027.00	2	.01	.00	IMP(SUB)	DAM
1	3	16.01	5027.00	3	5.00	12.40	IMP(SUB)	
1	4	21.01	4965.00	4	3.50	12.86	IMP(SUB)	
.
1	11	57.01	4736.00	11	2.00	3.50	IMP(SUB)	
1	12	59.01	4729.00	12	8.50	8.82	IMP(SUB)	
1	13	67.51	4654.00	13	8.00	6.63	IMP(SUB)	
1	14	75.51	4601.00					

WARNING: THE FOLLOWING DXMs SHOULD BE CHANGED

J	I	DXM(I, J)	RECOMMENDED	REASON
1	6	.750	.194	COURANT CONDITION
1	7	1.000	.264	COURANT CONDITION
1	8	1.000	.326	COURANT CONDITION
1	9	1.000	.202	COURANT CONDITION
1	10	1.100	.306	COURANT CONDITION
1	11	1.000	.222	EXP/CON CRITERIA
1	12	1.000	.425	COURANT CONDITION
1	13	1.400	.368	COURANT CONDITION

Definition of Variables in Bottom Slope Profile

- River No - River number
- Sect No - Cross section number
- X - Cross section location (MI or KM)
- Bed Elevation - Invert elevation (FT or M)
- Reach No - Reach number
- Length - Reach length (MI or KM)
- Slope - Slope of reach (FT/MI or PCT)
- Routing - Routing technique
- Struct. - Structure within the reach
- J - River number
- I - Cross section number
- DXM - Distance interval between cross sections (MI or KM)

Figure 6. Initial conditions summary

I	DISTANCE		FLOW		WSEL		DEPTH		MIN WSEL	BOTTOM
	MILE	CFS	FT	FT	FT	FT	FT			
1	.000	13000.	5288.499	68.499	5231.191	5220.000				
2	2.000	13000.	5288.500	92.625	5206.267	5195.875				
.										
.										
82	73.910	13000.	4621.069	9.469	4621.069	4611.600				
83	75.510	13000.	4609.491	8.491	4609.491	4601.000				

Definition of Variables in Initial Conditions Summary

- I - Cross section counter
- DISTANCE - Cross section location (MI or KM)
- FLOW - Initial discharge (CFS or CMS)
- WSEL - Initial water surface elevation (FT or M)
- DEPTH - Initial depth of flow (FT or M)
- MIN WSEL - Low flow filter (FT or M)
- BOTTOM - Invert elevation (FT or M)

Figure 7. Initial conditions/low flow filter

```

I= 1 X= .000 YN= 5288.55 DEPN= 261.55 YC= 5033.55 DEPC= 6.55 IFR= 0 ITN= 0 ITC= 14
I= 2 X= .010 YN= 5036.35 DEPN= 9.35 YC= 5033.55 DEPC= 6.55 IFR= 0 ITN= 14 ITC= 14
.
.
I= 73 X= 57.910 YN= 4621.22 DEPN= 9.62 YC= 4618.02 DEPC= 6.42 IFR= 0 ITN= 12 ITC= 12
I= 74 X= 59.510 YN= 4609.64 DEPN= 8.64 YC= 4606.61 DEPC= 5.61 IFR= 0 ITN= 12 ITC= 12
    
```

```

(IFR(I,J),I=1,N)
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0

WATER ELEVATION AT SECTION N= 74 IS 4609.49
WATER ELEVATION AT SECTION N= 73 IS 4621.07
    
```

```

BACKWATER      IN= 73      YNN= 4621.07      DEP= 9.47
I= 72 QIL= 13003. YIL= 4632.73 DEP= 10.53 ITB= 3
I= 71 QIL= 13003. YIL= 4644.56 DEP= 11.76 ITB= 3
.
.
I= 2 QIL= 13003. YIL= 5036.36 DEP= 9.36 ITB= 3
I= 1 QIL= 13003. YIL= 5288.55 DEP= 261.55 ITB= 3
    
```

INITIAL WATER ELEVATION:

```

YDI FOR RIVER NO. 1
5288.55 5036.36 5030.18 5024.00 5017.82 5011.63 5005.45 4999.26
4993.07 4986.87 4980.73 4974.40 4967.82 4961.25 4954.65 4948.10
4941.41 4935.00 4927.95 4921.06 4914.17 4907.29 4900.40 4893.51
4886.62 4879.75 4872.83 4866.01 4859.01 4852.35 4845.02 4839.00
4830.35 4827.67 4825.95 4824.23 4822.49 4820.75 4819.01 4817.24
4815.44 4813.59 4810.39 4806.91 4803.31 4799.70 4795.90 4792.09
4788.19 4784.44 4780.24 4776.87 4774.79 4772.68 4770.56 4768.40
4765.58 4760.03 4755.33 4750.90 4746.99 4741.20 4730.76 4720.34
4709.98 4699.59 4689.36 4678.84 4668.77 4656.56 4644.56 4632.73
4621.07 4609.49
    
```

WATER ELEVATION FOR LOW FILTER:

```

YUMN FOR RIVER 1
5036.36 5036.36 5030.18 5024.00 5017.82 5011.63 5005.45 4999.26
4993.07 4986.87 4980.73 4974.40 4967.82 4961.25 4954.65 4948.10
4941.41 4935.00 4927.95 4921.06 4914.17 4907.29 4900.40 4893.51
4886.62 4879.75 4872.83 4866.01 4859.01 4852.35 4845.02 4839.00
4830.35 4827.67 4825.95 4824.23 4822.49 4820.75 4819.01 4817.24
4815.44 4813.59 4810.39 4806.91 4803.31 4799.70 4795.90 4792.09
4788.19 4784.44 4780.24 4776.87 4774.79 4772.68 4770.56 4768.40
4765.58 4760.03 4755.33 4750.90 4746.99 4741.20 4730.76 4720.34
4709.98 4699.59 4689.36 4678.84 4668.77 4656.56 4644.56 4632.73
4621.07 4609.49
    
```

Definition of Variables in Initial Conditions/Low Flow Filter

- I - Cross section counter
- X - Cross section location (MI or KM)
- YN - Normal flow WSEL (FT or M) for initial flow at t=0
- DEPN - Normal flow depth (FT or M) for initial flow
- YC - Critical flow WSEL (FT or M) for initial flow at t=0
- DEPC - Critical flow depth (FT or M) for initial flow
- IFR - Froude number indicator 0 indicates $Fr < 1$, 1 indicates $Fr \geq 1$
- ITN - Number of iterations to obtain YN via bi-section solution method
- ITC - Number of iterations to obtain YC via bi-section solution method
- IN - Number of cross section at downstream boundary
- YNN - WSEL (FT or M) at downstream boundary for initial flow
- DEP - Depth (FT or M) at downstream boundary for initial flow
- I - Cross section counter
- J - River number
- QIL - Discharge (CFS or CMS) at t=0 for Ith cross section
- YIL - Computed backwater/downwater WSEL (FT or M) at t=0 for Ith cross section

DEP - Backwater flow depth (FT or M)
ITB - Number of iterations to obtain backwater elevation YIL
YDI - Initial water surface elevation (FT or M)
YUMN - Minimum water surface elevation (FT or M) used in routing
computations (low flow filter)

Figure 8. Minimum dynamic routing output

```

TT = .00000 HRS      DTH = .02500 HRS      ITMX= 0
RIVER= 1      QU(1)= 3.000      YU(1)= 2578.30      QU(N)= 3.000      YU(N)= 215.37
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 5.000 2578.30 .01 279.904 8060. 8060. 3.0000 .0700 .00 .00 120.30 10 .0000 0
1 2 5.010 2466.45 8.40 .357 85. 85. 3.0000 .0700 .00 .72 8.45 5 .0000 0
1 3 5.112 2454.16 8.38 .358 85. 85. 3.0000 .0700 .00 .72 8.43 5 .0000 0
1 4 5.214 2441.86 8.37 .359 85. 85. 3.0000 .0700 .00 .72 8.41 5 .0000 0
1 5 5.315 2429.56 8.35 .359 86. 86. 3.0000 .0700 .00 .72 8.38 5 .0000 0
1 6 5.417 2417.26 8.33 .360 86. 86. 3.0000 .0700 .00 .72 8.36 5 .0000 0
1 7 5.519 2404.96 8.32 .361 87. 87. 3.0000 .0700 .00 .72 8.33 5 .0000 0
1 8 5.621 2392.66 8.30 .361 87. 87. 3.0000 .0700 .00 .72 8.31 5 .0000 0
1 9 5.722 2380.36 8.28 .362 87. 87. 3.0000 .0700 .00 .72 8.28 5 .0000 0
FRMX= .873 IFRMX= 117 FRMN= .000 IFRMN= 1
RESERVOIR OUTFLOW INFORMATION
J I TT QU(I) USH(MSL) YB(MSL) DSH(MSL) SUB BB QU(1) QBRECH QOVTOP QOTHR
1 1 .000 3.000 2578.30 2578.30 2466.45 1.00 .00 3.000 .000 .000 3.000

```

Definition of Variables in Minimum Dynamic Routing Output

- TT - Time at which output is given (HR)
- DTH - Computational time step (HR)
- ITMX - Number of iterations in Newton-Raphson Solution of Saint-Venant Equations
- RIVER - River number
- QU(1) - Discharge (CFS or CMS) at upstream boundary
- YU(1) - Water surface elevation (FT or M) at upstream boundary
- QU(N) - Discharge (CFS or CMS) at downstream boundary
- YU(N) - Water surface elevation (FT or M) at downstream boundary
- FRMX - Maximum Froude number in the routing reach
- IFRMX - Cross section number at which FRMX occurs
- FRMN - Minimum Froude number in the routing reach
- IFRMN - Cross section number at which FRMN occurs

Figure 9. Internal boundary information

```

TT = .00000 HRS      DTH = .02500 HRS      ITMX= 0
RIVER= 1      QU(1)= 3.000      YU(1)= 2578.30      QU(N)= 3.000      YU(N)= 215.37
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 5.000 2578.30 .01 79.904 60. 60. 3.0000 .0700 .00 .00 120.30 10 .0000 0
1 2 5.010 2466.45 8.40 .357 85. 85. 3.0000 .0700 .00 .72 8.45 5 .0000 0
1 3 5.112 2454.16 8.38 .358 85. 85. 3.0000 .0700 .00 .72 8.43 5 .0000 0
1 4 5.214 2441.86 8.37 .359 85. 85. 3.0000 .0700 .00 .72 8.41 5 .0000 0
1 5 5.315 2429.56 8.35 .359 86. 86. 3.0000 .0700 .00 .72 8.38 5 .0000 0
1 6 5.417 2417.26 8.33 .360 86. 86. 3.0000 .0700 .00 .72 8.36 5 .0000 0
1 7 5.519 2404.96 8.32 .361 87. 87. 3.0000 .0700 .00 .72 8.33 5 .0000 0
1 8 5.621 2392.66 8.30 .361 87. 87. 3.0000 .0700 .00 .72 8.31 5 .0000 0
1 9 5.722 2380.36 8.28 .362 87. 87. 3.0000 .0700 .00 .72 8.28 5 .0000 0
FRMX= .873 IFRMX= 117      FRMN= .000 IFRMN= 1

```

```

RESERVOIR OUTFLOW INFORMATION
J I TT QU(I) USH(MSL) YB(MSL) DSH(MSL) SUB BB QU(1) QBRECH QOVTOP QOTHR
1 1 .000 3.000 2578.30 2578.30 2466.45 1.00 .00 3.000 .00 .000 3.000

```

Definition of Variables in Internal Boundary Information

- J - River number
- I - Cross section number of internal boundary
- TT - Time at which output is given (HR)
- QU(I) - Discharge through structure (CFS or CMS)
- USH(MSL) - Water surface elevation (FT or M above Mean Sea Level) immediately upstream of structure (pool elevation)
- YB(MSL) - Elevation (FT or M above Mean Sea Level) of bottom of breach
- DSH(MSL) - Water surface elevation (FT or M above Mean Sea Level) immediately downstream of structure (tailwater elevation)
- SUB - Submergence correction factor for breach flow
- BB - Bottom width (FT or M) of breach
- QU(1) - Discharge (CFS or CMS) at upstream end of the reach or pool upstream of the structure
- QBRECH - Discharge (CFS or CMS) through breach
- QOVTOP - Discharge (CFS or CMS) over the top of dam or over crest of bridge embankment
- QOTHR - Discharge (CFS or CMS) of all other flows (Dams: spillways, gates, turbines; Bridge: bridge opening)

Figure 10. Hydraulic information

```

TT = .00000 HRS      DTH = .02500 HRS      ITMX= 0
RIVER= 1      QU(1)= 3.000      YU(1)= 2578.30      QU(N)= 3.000      YU(N)= 215.37
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 5.000 2578.30 .01 79.904 60. 60. 3.0000 .0700 .00 .00 120.30 10 .0000 0
1 2 5.010 2466.45 8.40 .357 85. 85. 3.0000 .0700 .00 .72 8.45 5 .0000 0
1 3 5.112 2454.16 8.38 .358 85. 85. 3.0000 .0700 .00 .72 8.43 5 .0000 0
1 4 5.214 2441.86 8.37 .359 85. 85. 3.0000 .0700 .00 .72 8.41 5 .0000 0
1 5 5.315 2429.56 8.35 .359 86. 86. 3.0000 .0700 .00 .72 8.38 5 .0000 0
1 6 5.417 2417.26 8.33 .360 86. 86. 3.0000 .0700 .00 .72 8.36 5 .0000 0
1 7 5.519 2404.96 8.32 .361 87. 87. 3.0000 .0700 .00 .72 8.33 5 .0000 0
1 8 5.621 2392.66 8.30 .361 87. 87. 3.0000 .0700 .00 .72 8.31 5 .0000 0
1 9 5.722 2380.36 8.28 .362 87. 87. 3.0000 .0700 .00 .72 8.28 5 .0000 0
FRMX= .873 IFRMX= 117 FRMN= .000 IFRMN= 1
RESERVOIR OUTFLOW INFORMATION
J I TT QU(I) USH(MSL) YB(MSL) DSH(MSL) SUB BB QU(1) QBRECH QOVTOP QOTHR
1 1 .000 3.000 2578.30 2578.30 2466.45 1.00 .00 3.000 .00 .000 3.000

```

Definition of Variables in Hydraulic Information

- J - River number
- I - Cross section number
- X(MI) - Cross section location (MI of KM)
- H(MSL) - Water surface elevation (FT or M above Mean Sea Level)
- V(FPS) - Velocity (FT/SEC or M/SEC)
- A(TSQFT) - Active cross sectional area (1000 FT² or 1000 MI²)
- B(FT) - Active topwidth (FT or M)
- BT(FT) - Total topwidth (FT or M)
- Q(TCFS) - Discharge (1000 CFS or 1000 CMS)
- MANN. N - Roughness coefficient
- WAVHT - Wave height (FT or M) - H minus initial WSEL
- FROUDE - Froude number
- DEP(FT) - Water depth (FT or M) - H minus invert elevation
- KR - KRCH routing/internal boundary type parameter
- QL(TCFS) - Lateral flow (1000 CFS or 1000 CMS)
- MRV - River into which tributary flows

Figure 11. Levee information

```

TT = .50000 HRS      DTH = .50000 HRS      ITMX= 1 1
RIVER= 1  QU(1)= 3.354  YU(1)= 109.37  QU(N)= 5.994  YU(N)= 71.83

J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 .000 109.37 1.53 2.193 468. 468. 3.3542 .0400 .16 .12 9.37 0 .0000 0
1 2 5.000 104.14 11.36 2.088 457. 457. 2.8485 .4000 -.07 .11 9.14 0 .0000 0
1 3 10.000 99.25 1.43 2.140 463. 463. 3.0649 .0400 .03 .12 9.25 9 .0000 0
1 4 11.250 98.00 1.41 2.137 462. 462. 3.0151 .0400 .01 .12 9.25 9 .0000 0
1 5 12.500 96.77 1.40 2.150 464. 464. 3.0035 .0400 .00 .11 9.27 9 .0000 0
1 6 13.750 95.59 1.38 2.181 467. 467. 3.0008 .0400 .00 .11 9.34 9 .0000 0
1 7 15.000 94.48 1.34 2.246 474. 474. 3.0002 .0400 .00 .11 9.48 9 .0000 0
1 8 17.500 92.80 1.13 2.649 510. 510. 3.0000 .0400 .00 .09 10.30 9 .0000 0
1 9 20.000 91.93 .85 3.526 564. 564. 3.0000 .0400 .00 .06 11.93 0 .0000 0
1 0 20.100 91.80 1.74 3.452 560. 560. 6.0000 .0400 .00 .12 11.80 0 .0000 0
1 11 25.000 86.84 1.72 3.479 561. 561. 6.0000 .0400 .00 .12 11.84 9 .0000 0
1 12 26.000 85.84 1.72 3.479 561. 561. 6.0000 .0400 .00 .12 11.84 9 .0000 0
1 13 27.000 84.84 1.72 3.479 561. 561. 6.0000 .0400 .00 .12 11.84 9 .0000 0
1 14 28.000 83.84 1.72 3.479 561. 561. 5.9999 .0400 .00 .12 11.84 9 .0000 0
1 15 29.000 82.84 1.72 3.479 561. 561. 5.9996 .0400 .00 .12 11.84 9 .0000 0
1 16 30.000 81.85 1.72 3.479 562. 562. 5.9982 .0400 .00 .12 11.85 0 .0000 0
1 17 35.000 76.84 1.73 3.479 561. 561. 6.0034 .0400 .00 .12 11.84 0 .0000 0
1 18 40.000 71.83 1.73 3.473 561. 561. 5.9937 .0400 .00 .12 11.83 0 .0000 0

FRMX= .125  IFRMX= 1  FRMN= .060
RIVER= 2  QU(2)= 3.063  YU(1)= 109.24  QU(N)= 3.000  YU(N)= 91.86

J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
2 1 .000 109.24 1.44 2.133 462. 462. 3.0625 .0400 .03 .12 9.24 0 .0000 1
2 2 5.000 104.20 1.41 2.115 460. 460. 2.9732 .0400 .01 .12 9.20 9 .0000 1
2 3 7.500 101.71 1.42 2.121 460. 460. 3.0030 .0400 .00 .12 9.21 9 .0000 1
2 4 10.000 99.21 1.42 2.119 460. 460. 2.9997 .0400 .00 .12 9.21 9 .0000 1
2 5 11.250 97.95 1.42 2.117 460. 460. 2.9999 .0400 .00 .12 9.20 9 .0000 1
2 6 12.500 96.69 1.42 2.113 460. 460. 3.0000 .0400 .00 .12 9.19 9 .0000 1
2 7 13.750 95.43 1.42 2.105 459. 459. 3.0000 .0400 .00 .12 9.18 9 .0000 1
2 8 15.000 94.14 1.44 2.088 457. 457. 3.0000 .0400 .00 .12 9.14 0 .0000 1
2 9 20.000 91.86 .86 3.489 562. 562. 3.0000 .0400 .00 .06 11.86 0 .0000 1

FRMX= 118  IFRMX= 8  FRMN= .061  IFRMN= 9
TT LV JM IM JT IT QLOVTP QLPOND QLBRCH BR-WDTH WSEL-M WSEL-T SUB-M SUB-T
0.500 9 1 7 1 0 0.000 75.708 0.000 0.000 93.637 94.000 1.00 1.00
0.500 10 1 8 1 0 0.000 160.862 0.000 0.000 92.361 94.000 1.00 1.00

QPOND(L)= -237. 0. 0.
HPOND(L)= 93.99 85.00 70.00
    
```

Definition of Variables in Levee Information

- L - Pond counter
- QPOND(L) - Discharge into (+) or leaving (-) pond
- HPOND(L) - WSEL in pond (FT or M)
- LV - Levee reach number
- JM - Number of river passing flow over levee reach LV
- IM - Cross section reach number on river JM passing flow over levee reach LV
- JT - Number of river receiving flow from levee reach LV
- IT - Cross section reach number on river JT receiving flow from levee reach LV
- QLOVTP - Flow over the levee (CFS or CMS)
- QLBRCH - Flow through the levee breach (CFS or CMS)
- QLPOND - Flow from the pond (CFS or CMS)
- BR-WDTH - Width of levee breach (FT or M)
- WSEL-M - Average WSEL in reach IM (pool)
- WSEL-T - Average WSEL in reach IT (tailwater)
- SUB-M - Submergence correction factor for the main river
- SUB-T - Submergence correction factor for the tributary

Figure 12. Subcritical/Supercritical flow information

```

L= 1 KSP= 0 KS1= 1 KSN= 12
L= 2 KSP= 1 KS1= 12 KSN= 13
L= 3 KSP= 0 KS1= 14 KSN= 24

TT = .00000 HRS DTH = .01250 HRS ITMX= 0
RIVER= 1 QU(1)= .804 YU(1)= 549.98 QU(N)= 1.204 YU(N)= 466.81

J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 10 2.001 506.49 5.71 .211 65. 65. 1.2040 .0400 .00 .56 6.49 0 .0000 0
1 11 2.250 502.56 4.22 .285 76. 76. 1.2040 .0400 .00 .38 7.56 0 .0000 0
1 12 2.500 495.14 9.11 .132 51. 51. 1.2040 .0400 .00 1.00 5.14 0 .0000 0
1 13 2.600 477.02 1.09 .109 43. 43. 1.2040 .0400 .00 1.23 5.02 0 .0000 0
1 14 2.700 477.02 .60 .007 74. 74. 1.2040 .0400 .00 .03 23.02 0 .0000 0
1 15 2.800 477.02 .21 .684 77. 77. 1.2040 .0400 .00 .01 41.02 0 .0000 0
1 16 2.900 477.02 .11 .620 60. 60. 1.2040 .0400 .00 .00 59.02 0 .0000 0
1 17 3.000 477.02 .07 .478 28. 28. 1.2040 .0400 .00 .00 77.02 0 .0000 0
1 18 3.100 477.02 .10 .108 84. 84. 1.2040 .0400 .00 .00 63.02 0 .0000 0
1 19 3.200 477.02 .15 .118 31. 31. 1.2040 .0400 .00 .01 49.02 0 .0000 0

FRMX= 1.234 IFRMX= 1 FRMN= .002 IFRMN= 17

RESERVOIR OUTFLOW INFORMATION
J I TT QU(I) USH(MSL) YB(MSL) DSH(MSL) SUB BB QU(1) QBRECH QOVTOP QOTHR
1 9 .000 1.204 549.90 550.00 506.49 1.00 .00 .804 .000 .000 1.204

```

Definition of Variables in Subcritical/Supercritical Flow Information

- L - Flow regime reach counter
- KSP - Flow regime indicator: 0 for subcritical flow, 1 for supercritical flow
- KS1 - Beginning cross section in flow regime
- KSN - Ending cross section in flow regime

Figure 13. Nonconvergence information

```

L= 1 KSP= 0 KS1= 1 KSN= 2
L= 2 KSP= 1 KS1= 2 KSN= 16
L= 3 KSP= 0 KS1= 17 KSN= 36
L= 4 KSP= 1 KS1= 36 KSN= 42
L= 5 KSP= 0 KS1= 43 KSN=112

NONCONVERGENCE FOR TT= .08300 USING DTH= .00415
AT RIVER= 1 SECT NO.= 17 18 19 20
PREVIOUS TT= .07885 NEW DTH= .00207 NEW F1= 1.00 NEW TT= .08093

L= 1 KSP= 0 KS1= 1 KSN= 2
L= 2 KSP= 1 KS1= 2 KSN= 17
L= 3 KSP= 0 KS1= 18 KSN= 36
L= 4 KSP= 1 KS1= 36 KSN= 42
L= 5 KSP= 0 KS1= 43 KSN=112

TT = .08093 HRS DTH = .00207 HRS ITMX= 4
RIVER= 1 QU(1)= 77.897 YU(1)= 1527.29 QU(N)= 1.002 YU(N)= 720.18

J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 .000 1527.29 22.15 3.517 215. 215. 77.8972 .0350 -21.71 .97 22.29 10 .0000 0
1 2 .010 1523.53 27.32 2.852 243. 243. 77.8972 .0350 14.97 1.40 18.53 0 .0000 0
1 3 .064 1568.06 28.80 2.819 241. 251. 81.1905 .0350 15.20 1.48 17.42 0 .0000 0
1 4 .117 1512.68 29.67 2.797 240. 261. 82.9958 .0350 14.43 1.53 16.40 0 .0000 0
1 5 .171 1507.33 30.04 2.774 241. 271. 83.3153 .0350 13.91 1.56 15.42 0 .0000 0
1 6 .225 1502.02 30.01 2.749 243. 283. 82.4936 .0350 13.00 1.57 14.48 0 .0000 0
1 7 .278 1496.71 29.76 2.710 245. 295. 80.6580 .0350 12.40 1.58 13.53 0 .0000 0
1 8 .332 1491.41 29.32 2.660 249. 307. 77.9996 .0350 11.40 1.58 12.59 0 .0000 0
1 9 .385 1486.11 28.75 2.595 253. 320. 74.5900 .0350 10.70 1.58 11.66 0 .0000 0
FRMX= 1.582 IFRMX= 9 FRMN= .282 IFRMN= 101

RESERVOIR OUTFLOW INFORMATION
J I TT QU(I) USH(MSL) YB(MSL) DSH(MSL) SUB BB QU(1) QBRECH QOVTOP QOTHR
1 1 .081 77.897 1527.29 1506.10 1523.53 .86 165.75 77.897 77.877 .000 .000

```

Definition of Variables in Nonconvergence Information

- SECT NO. - Cross locations (interpolated) where nonconvergence occurred.
- TT - Last computational time prior to nonconvergence
- NEW DTH - New computation time step (HR) to be used (usually half of the previous time step)
- NEW F1 - New theta weighting factor to be used.
- NEW TT - New time (HR) for which computations are made

Figure 14. Calibration information

```

RIVER NO. 1      REACH NO. 5      METROPOLIS      STA NO. 13      RIVER MILE= 991.200
RIVER NO. 1      MANNING N REACH NO. 5
FKC= 1000.0000    FMC= 0.5000      FKF= 400.0000    FMF= 1.0000      FKO= 0.0000      FMO= 0.0000
X(I,J)           991.20          981.70          972.20
IFXC=            0              0              0
HS= 271.56       281.56         321.56         325.56         341.56         441.56
BS= 0.00         3162.28       7071.07       8671.07       15071.07      15071.07
AS= 0.           15811.        220478.       251963.       441900.       1949007.

MCM= 1          TOTAL RMS (SEA)= 1.91      TOTAL MEAN DEVIATION (AVD)= 0.99
M              IIM      RMSL      AVDL      CM          YQR
1              0        0.0000    0.0000    0.0210     150000.
2              5        0.7607    0.7569    0.0210     250000.
3              18       1.4101    0.8834    0.0210     350000.
4              29       2.9710    2.7012    0.0210     450000.
5              6        0.2570    -0.0209   0.0210     550000.
6              6        0.5158    -0.3314   0.0210     650000.
7              20       0.7275    -0.6556   0.0210     750000.
8              0        0.0000    0.0000    0.0210     10000000000.

NEW CM=        0.0210    0.0194    0.0191    0.0168    0.0210    0.0217    0.0224    0.0210
    
```

Definition of Variables in Calibration Information

- FKC - Scaling parameter of in-bank channel portion of cross section
- FMC - Shape factor for in-bank channel portion of cross section
- FKF - Scaling parameter of floodplain portion of cross section
- FMF - Shape factor for floodplain portion of cross section
- FKO - Scaling parameter of inactive portion of cross section
- FMO - Shape factor for inactive portion of cross section
- X(I,J) - Cross section location (MI or KM)
- IFXC - Parameter indicating if cross sections have special properties
- HS - Elevation (FT or M) corresponding to each topwidth
- BS - Topwidth (FT or M) of active flow portion of cross section
- AS - Cross-sectional area (FT² or M²) below the corresponding HS value
- MCM - Iteration counter of each new calibration trial in the reach
- M - Level in Manning n table
- IIM - No. of hydrograph points in this level
- RMSL - Root-mean-squared (RMS) error for points in the level
- AVDL - Mean deviation of points in this level
- CM - Manning n value used in this range
- YQR - Average discharge/water surface elevation is this range

Figure 15. Profile of crests and times

RVR NO.	SEC NO.	LOCATION MILE	BOTTOM FEET	TIME MAX WSEL (HR)	MAX WSEL FEET	TIME MAX FLOW (CFS)	MAX FLOW CFS	MAX VL (FPS)	MAX VC (FPS)	MAX VR (FPS)
1	1*#	0.000	500.10	7.20002	551.04	7.57502	100976.	3.18	3.66	3.17
1	2*	0.010	500.00	7.85002	533.03	7.57502	100976.	7.73	14.07	7.73
1	3	0.111	499.50	7.90002	532.42	7.60002	98258.	7.57	13.46	7.57
1	4	0.212	498.99	7.92502	531.81	7.60002	95726.	7.37	12.98	7.37
1	5	0.313	498.49	7.95002	531.20	7.67502	93227.	7.17	12.54	7.17
1	6	0.413	497.98	7.97502	530.62	7.70002	91650.	7.01	12.25	7.01
1	7	0.514	497.48	8.00002	530.04	7.72502	90403.	6.92	11.88	6.92
1	8	0.615	496.97	8.05002	529.48	7.75002	89353.	6.84	11.53	6.84
1	9	0.716	496.47	8.25002	528.94	7.77502	88337.	6.75	11.19	6.75
1	10	0.817	495.97	8.27502	528.48	7.80002	87360.	6.74	10.57	6.74
1	11	0.918	495.46	8.30002	528.02	7.82502	86456.	6.64	10.16	6.64
		.								
		.								
		.								
1	77	7.600	469.80	9.87499	498.98	9.50000	59009.	4.72	9.21	4.72
1	78	7.800	469.40	9.89999	498.39	9.52500	58782.	4.74	9.23	4.74
1	79	8.000	469.00	9.92499	497.79	9.57500	58553.	4.73	9.23	4.73
1	80	8.200	468.60	9.94999	497.21	9.60000	58314.	4.72	9.20	4.72
1	81	8.400	468.20	9.97499	496.59	9.65000	58072.	4.66	9.18	4.66
1	82	8.600	467.80	9.97499	495.88	9.67500	57846.	4.23	9.29	4.23
1	83	8.800	467.40	9.99999	495.11	9.72499	57622.	4.05	9.32	4.05
1	84	9.000	467.00	10.02499	494.22	9.77499	57394.	4.03	9.28	4.03
1	85	9.200	466.60	10.07499	493.27	9.79999	57160.	4.05	9.21	4.05
1	86	9.400	466.20	10.09999	492.25	9.87499	56931.	4.10	9.10	4.10
1	87	9.600	465.80	10.12499	491.15	9.92499	56728.	4.21	8.93	4.21
1	88	9.800	465.40	10.17499	490.01	9.99999	56539.	4.26	8.79	4.26
1	89*	10.000	465.00	10.22499	488.84	10.04999	56366.	4.38	8.51	4.38

Definition of Variables in Profile of Crests and Times

- RVR NO. - River number
- SEC NO. - Cross section number
- LOCATION - Cross section location (MI or KM)
- BOTTOM - Invert elevation (FT or M)
- TIME MAX WSEL - Time to maximum water surface elevation (hr)
- MAX WSEL - Maximum water surface elevation (FT or M)
- TIME MAX FLOW - Time to maximum flow (HR)
- MAX FLOW - Maximum flow (CFS or CMS)
- MAX VL - Maximum flow velocity in the left floodplain (FT/SEC or M/SEC) (floodplain option only)
- MAX VC - Maximum flow velocity in the channel (FT/SEC or M/SEC) (floodplain option only)
- MAX VR - Maximum flow velocity in the right floodplain (FT/SEC or M/SEC) (floodplain option only)
- MAX VEL - Maximum flow velocity in the composite channel (FT/SEC or M/SEC) (composite channel option only)

Figure 16. Computed water surface elevation and discharges

KTIME	TII(KTIME)	COMPUTED STAGES FOR RIVER= 1 SECTION= 1 2 43 112			
1	.000	1549.00	1508.55	1202.32	720.18
2	.008	1548.99	1509.50	1202.33	720.18
3	.017	1548.88	1511.81	1202.35	720.18
4	.025	1548.53	1514.23	1202.36	720.18
5	.033	1547.77	1516.33	1202.36	720.18
6	.041	1546.44	1518.31	1202.36	720.18
7	.050	1544.36	1520.23	1202.36	720.18
8	.058	1541.37	1521.92	1202.36	720.18
9	.066	1537.29	1523.36	1202.36	720.18
10	.075	1531.79	1524.90	1202.36	720.18
11	.083	1525.96	1522.33	1202.36	720.18
12	.091	1522.53	1518.49	1202.36	720.18
13	.100	1520.06	1517.47	1202.36	720.18

KTIME	TII(KTIME)	COMPUTED DISCHARGE FOR RIVER= 1 SECTION= 1 2 43 112			
1	.000	1000.	1000.	1001.	1000.
2	.008	1741.	1741.	1021.	1000.
3	.017	5114.	5114.	1041.	1001.
4	.025	11700.	11700.	1053.	1001.
5	.033	21477.	21477.	1058.	1001.
6	.041	33930.	33930.	1060.	1001.
7	.050	48255.	48255.	1061.	1001.
8	.058	62646.	62646.	1061.	1001.
9	.066	76294.	76294.	1062.	1002.
10	.075	92148.	92148.	1062.	1002.
11	.083	66385.	66385.	1062.	1002.
12	.091	35124.	35124.	1061.	1002.
13	.100	28365.	28365.	1061.	1002.

Definition of Variables in Computed Water Surface Elevation and Discharges

- KTIME - Time step counter
- TII(KTIME) - Time (HR) at which computed stages and computed discharges for each river occur.
- SECTION - Number of cross sections
- YC(KTIME,I) - Water surface elevation (FT or M) for each time at each station where hydrograph plot is made
- QC(KTIME,I) - Discharge (CFS or CMS) for each time at each station where hydrograph plot is made

Figure 17. Initial conditions/low flow filter - normal depth computations

```

I= 1  X= .000  YN= 5288.55  DEPN= 261.55  YC= 5033.55  DEPC= 6.55  IFR= 0  ITN= 0  ITC= 14
I= 2  Y= 5076.00  F= 684119.8  FK= .0722  A= 35049.91  R= 36.57  CMU= .0400  Q1= 13003.33
I= 2  Y= 5051.50  F= 139161.1  FK= .0722  A= 13230.69  R= 16.08  CMU= .0400  Q1= 13003.33
I= 2  Y= 5039.25  F= 15225.0  FK= .0722  A= 4319.08  R= 6.89  CMU= .0400  Q1= 13003.33
I= 2  Y= 5033.13  F= -8790.1  FK= .0722  A= 1106.71  R= 3.06  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.19  F= -581.4  FK= .0722  A= 2490.10  R= 4.59  CMU= .0400  Q1= 13003.33
I= 2  Y= 5037.72  F= 6259.5  FK= .0722  A= 3378.31  R= 5.61  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.95  F= 2374.2  FK= .0722  A= 2922.41  R= 4.98  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.57  F= 847.2  FK= .0722  A= 2701.93  R= 4.79  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.38  F= 120.7  FK= .0722  A= 2594.93  R= 4.69  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.28  F= -233.4  FK= .0722  A= 2542.25  R= 4.64  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.33  F= -57.1  FK= .0722  A= 2568.52  R= 4.67  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.35  F= 31.6  FK= .0722  A= 2581.71  R= 4.68  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.34  F= -13.7  FK= .0722  A= 2574.98  R= 4.67  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.35  F= 8.1  FK= .0722  A= 2578.21  R= 4.67  CMU= .0400  Q1= 13003.33
I= 2  Y= 5036.35  F= -2.8  FK= .0722  A= 2576.59  R= 4.67  CMU= .0400  Q1= 13003.33
.
.
I= 74  Y= 4610.50  F= 3243.9  FK= .0528  A= 3143.75  R= 6.62  CMU= .0360  Q1= 13003.33
I= 74  Y= 4605.75  F= -10606.5  FK= .0528  A= 953.20  R= 2.25  CMU= .0360  Q1= 13003.33
I= 74  Y= 4608.13  F= -5008.5  FK= .0528  A= 2031.29  R= 4.40  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.31  F= -1178.6  FK= .0528  A= 2583.60  R= 5.52  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.91  F= 960.6  FK= .0528  A= 2862.70  R= 6.07  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.61  F= -127.2  FK= .0528  A= 2722.91  R= 5.79  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.76  F= 412.2  FK= .0528  A= 2792.74  R= 5.93  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.68  F= 141.3  FK= .0528  A= 2757.81  R= 5.86  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.65  F= 6.8  FK= .0528  A= 2740.35  R= 5.83  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.63  F= -60.3  FK= .0528  A= 2731.63  R= 5.81  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.64  F= -26.8  FK= .0528  A= 2735.99  R= 5.82  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.64  F= -10.9  FK= .0528  A= 2738.06  R= 5.82  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.64  F= -2.1  FK= .0528  A= 2739.21  R= 5.83  CMU= .0360  Q1= 13003.33
I= 74  X= 59.510  YN= 4609.64  DEPN= 8.64  YC= 4606.61  DEPC= 5.61  IFR= 0  ITN= 12  ITC= 12

```

Definition of Variables in Initial Conditions/Low Flow Filter - Normal Depth Computations

- I - Cross section counter
- X - Cross section location (MI or KM)
- YN - Normal flow WSEL (FT or M) for initial flow at t=0
- DEPN - Normal flow depth (FT or M) for initial flow
- YC - Critical flow WSEL (FT or M) for initial flow at t=0
- DEPC - Critical flow depth (FT or M) for initial flow
- IFR - Froude number indicator 0 indicates $Fr < 1$, 1 indicates $Fr \geq 1$
- ITN - Number of iterations to obtain YN via bi-section solution method
- ITC - Number of iterations to obtain YC via bi-section solution method
- J - River number
- N - Total number of cross sections
- Y - Water surface elevation (FT or M)
- F - Difference between the computed discharge and the actual discharge
- FK - $1.49 \cdot \text{SQRT}(S)$
- A - Active cross sectional area (FT² or M²)
- R - Hydraulic Radius (FT or M)
- CMU - Manning Roughness Coefficient
- Q1 - Discharge (CFS or CMS)

Figure 18. Initial conditions/low flow filter - downwater computations

```

I= 1  X= .000  YN= 5288.55  DEPN= 261.55  YC= 5033.55  DEPC= 6.55  IFR= 0  ITN= 0  ITC= 14
I= 2  Y= 5076.00  F= 684119.8  FK= .0722  A= 35049.91  R= 36.57  CMU= .0400  Q1= 13003.33
I= 2  Y= 5051.50  F= 139161.1  FK= .0722  A= 13230.69  R= 16.08  CMU= .0400  Q1= 13003.33
I= 2  Y= 5039.25  F= 15225.0  FK= .0722  A= 4319.08  R= 6.89  CMU= .0400  Q1= 13003.33
.
.
I= 74  Y= 4609.64  F= -26.8  FK= .0528  A= 2735.99  R= 5.82  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.64  F= -10.9  FK= .0528  A= 2738.06  R= 5.82  CMU= .0360  Q1= 13003.33
I= 74  Y= 4609.64  F= -2.1  FK= .0528  A= 2739.21  R= 5.83  CMU= .0360  Q1= 13003.33
I= 74  X= 59.510  YN= 4609.64  DEPN= 8.64  YC= 4606.61  DEPC= 5.61  IFR= 0  ITN= 12  ITC= 12

(IFR(I, J), I=1, N)
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
0 0 0 0 0 0 0 0 0 0
ITB= 0  I= 73  YIR= 4609.64  QII= 13003.  YA= 4630.56  F= -3015074.000
ITB= 1  I= 73  YIR= 4609.64  QII= 13003.  YA= 4624.29  F= -1026617.000
ITB= 2  I= 73  YIR= 4609.64  QII= 13003.  YA= 4621.16  F= -59822.660
.
.
ITB= 11  I= 73  YIR= 4609.64  QII= 13003.  YA= 4620.98  F= -549.270
ITB= 12  I= 73  YIR= 4609.64  QII= 13003.  YA= 4620.98  F= 602.136

WATER ELEVATION AT SECTION N= 74 IS 4609.49
WATER ELEVATION AT SECTION N= 73 IS 4621.07

BACKWATER  IN= 73  YNN= 4621.07  DEP= 9.47
ITB= 1  I= 72  YBWO= 4631.41  YBWN= 4632.64  F= 460831.400
ITB= 2  I= 72  YBWO= 4632.64  YBWN= 4632.73  F= 31074.660
ITB= 3  I= 72  YBWO= 4632.73  YBWN= 4632.73  F= 106.291
I= 72  QIL= 13003.  YIL= 4632.73  DEP= 10.53  ITB= 3
ITB= 1  I= 71  YBWO= 4642.95  YBWN= 4644.80  F= 528940.500
ITB= 2  I= 71  YBWO= 4644.80  YBWN= 4644.56  F= -73286.410
ITB= 3  I= 71  YBWO= 4644.56  YBWN= 4644.56  F= 687.131
I= 71  QIL= 13003.  YIL= 4644.56  DEP= 11.76  ITB= 3
ITB= 1  I= 70  YBWO= 4654.70  YBWN= 4656.77  F= 538055.800
ITB= 2  I= 70  YBWO= 4656.77  YBWN= 4656.56  F= -64059.640
ITB= 3  I= 70  YBWO= 4656.56  YBWN= 4656.56  F= 411.615
.
.
I= 3  QIL= 13003.  YIL= 5030.18  DEP= 9.38  ITB= 3
ITB= 1  I= 2  YBWO= 5035.55  YBWN= 5036.38  F= 169358.600
ITB= 2  I= 2  YBWO= 5036.38  YBWN= 5036.36  F= -4883.037
ITB= 3  I= 2  YBWO= 5036.36  YBWN= 5036.36  F= -52.562
I= 2  QIL= 13003.  YIL= 5036.36  DEP= 9.36  ITB= 3
I= 1  QIL= 13003.  YIL= 5288.55  DEP= 261.55  ITB= 3

```

Definition of Variables in Initial Conditions/Low Flow Filter - Downwater Computations

- I - Cross section counter
- F - Difference between the computed discharge and the actual discharge
- YIR - Final water surface elevation (FT or M)
- QII - Discharge (same as Q1) (CFS or CMS)
- YA - Water surface elevation within the reach (FT or M)

Figure 19. Initial conditions/low flow filter - backwater computations

```

ITB= 0   I= 73   YIR= 4609.64   QII= 13003.   YA= 4630.56   F= -3015074.000
ITB= 1   I= 73   YIR= 4609.64   QII= 13003.   YA= 4624.29   F= -1026617.000
ITB= 2   I= 73   YIR= 4609.64   QII= 13003.   YA= 4621.16   F= -59822.660
.
.
ITB= 11  I= 73   YIR= 4609.64   QII= 13003.   YA= 4620.98   F= -549.270
ITB= 12  I= 73   YIR= 4609.64   QII= 13003.   YA= 4620.98   F= 602.136

WATER ELEVATION AT SECTION N= 74 IS 4609.49
WATER ELEVATION AT SECTION N= 73 IS 4621.07

BACKWATER      IN= 73      YNN= 4621.07      DEP= 9.47
ITB= 1   I= 72   YBWO= 4631.41   YBWN= 4632.64   F= 460831.400
ITB= 2   I= 72   YBWO= 4632.64   YBWN= 4632.73   F= 31074.660
ITB= 3   I= 72   YBWO= 4632.73   YBWN= 4632.73   F= 106.291
I= 72   QIL= 13003.   YIL= 4632.73   DEP= 10.53   ITB= 3
ITB= 1   I= 71   YBWO= 4642.95   YBWN= 4644.80   F= 528940.500
ITB= 2   I= 71   YBWO= 4644.80   YBWN= 4644.56   F= -73286.410
ITB= 3   I= 71   YBWO= 4644.56   YBWN= 4644.56   F= 687.131
I= 71   QIL= 13003.   YIL= 4644.56   DEP= 11.76   ITB= 3
ITB= 1   I= 70   YBWO= 4654.70   YBWN= 4656.77   F= 538055.800
ITB= 2   I= 70   YBWO= 4656.77   YBWN= 4656.56   F= -64059.640
ITB= 3   I= 70   YBWO= 4656.56   YBWN= 4656.56   F= 411.615
.
.
I= 3   QIL= 13003.   YIL= 5030.18   DEP= 9.38   ITB= 3
ITB= 1   I= 2   YBWO= 5035.55   YBWN= 5036.38   F= 169358.600
ITB= 2   I= 2   YBWO= 5036.38   YBWN= 5036.36   F= -4883.037
ITB= 3   I= 2   YBWO= 5036.36   YBWN= 5036.36   F= -52.562
I= 2   QIL= 13003.   YIL= 5036.36   DEP= 9.36   ITB= 3
I= 1   QIL= 13003.   YIL= 5288.55   DEP= 261.55   ITB= 3

INITIAL WATER ELEVATION:
YDI FOR RIVER NO. 1
5288.55  5036.36  5030.18  5024.00  5017.82  5011.63  5005.45  4999.26
5993.07  4986.87  4980.73  4974.40  4967.82  4961.25  4954.65  4948.10
5941.41  4935.00  4927.95  4921.06  4914.17  4907.29  4900.40  4893.51
5886.62  4879.75  4872.83  4866.01  4859.01  4852.35  4845.02  4839.00
5830.35  4827.67  4825.95  4824.23  4822.49  4820.75  4819.01  4817.24
5815.44  4813.59  4810.39  4806.91  4803.31  4799.70  4795.90  4792.09
5788.19  4784.44  4780.24  4776.87  4774.79  4772.68  4770.56  4768.40
5765.58  4760.03  4755.33  4750.90  4746.99  4741.20  4730.76  4720.34
5709.98  4699.59  4689.36  4678.84  4668.77  4656.56  4644.56  4632.73
5621.07  4609.49

```

Definition of Variables in Initial Conditions/Low Flow Filter - Backwater Computations

- I - Cross section counter
- F - Difference between the computed discharge and the actual discharge
- IN - Number of cross section at downstream boundary
- YNN - WSEL (FT or M) at downstream boundary for initial flow
- DEP - Depth (FT or M) at downstream boundary for initial flow
- QIL - Discharge (CFS or CMS) at t=0 for Ith cross section
- YIL - Computed backwater/downwater WSEL (FT or M) at t=0 for Ith cross section
- DEP - Backwater flow depth (FT or M)
- ITB - Number of iterations to obtain backwater elevation YIL
- YDI - Initial water surface elevation (FT or M)
- YUMN - Minimum water surface elevation (FT or M) used in routing computations (low flow filter)
- YBWN - New guess for the water surface elevation (FT or M)
- YBWO - Old guess for the water surface elevation (FT or M)

Figure 20. Outflow summary

```

TT = 48.00000 HRS      DTH = 24.00000 HRS      ITMX= 2 1 1 1
RIVER= 1      QU(1)= 330.000      YU(1)= 337.15      QU(N)= 619.472      YU(N)= 257.99

J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 1076.500 337.15 3.65 90.463 3182. 3182. 330.0000 .0294 1.58 .12 37.15 0 .0000 0
1 2 1067.300 334.30 3.07 07.291 4775. 4775. 328.9608 .0294 2.15 .11 34.30 0 .0160 0
1 3 1058.000 330.88 3.71 89.024 3845. 3845. 330.2678 .0229 2.18 .14 30.88 0 .0000 0
1 4 1049.900 328.98 3.57 91.995 3587. 3587. 328.6409 .0229 2.77 .12 33.98 0 .0000 0
1 5 1031.700 322.86 4.54 71.678 3924. 3924. 325.5422 .0199 2.26 .19 22.86 0 .0000 0
.
.
.
1 19 937.400 298.45 4.25 48.958 5770. 5770. 632.6872 .0237 4.45 .15 38.45 0 .0000 0
1 20 920.000 291.49 5.04 25.068 5280. 5280. 630.1964 .0237 4.07 .18 31.49 0 .0000 0
1 21 904.500 283.91 4.92 27.549 5411. 5411. 627.2339 .0237 2.36 .18 33.91 0 .0000 0
1 22 889.000 278.75 4.15 50.285 4943. 4943. 623.9164 .0258 2.07 .13 38.75 0 .0000 0
1 23 867.700 270.83 4.11 51.008 7100. 7100. 621.2112 .0258 5.09 .16 30.83 0 .0000 0
1 24 846.400 257.99 5.40 14.722 5492. 5492. 619.4720 .0258 2.20 .21 17.99 0 .0000 0

FRMX= .263 IFRMX= 7      FRMN= .112 IFRMN= 15
RIVER= 2      QU(1)= 53.800      YU(1)= 320.50      QU(N)= 53.270      YU(N)= 315.36
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
2 1 30.600 320.50 3.14 17.129 728. 728. 53.8000 .0213 1.36 .11 20.50 0 .0000 1
2 2 23.000 319.30 3.01 17.831 819. 819. 53.7196 .0213 1.33 .11 19.30 0 .0000 1
2 3 15.300 317.88 3.23 16.587 823. 823. 53.6077 .0213 1.09 .13 17.88 0 .0000 1
2 4 7.700 316.47 3.15 16.956 759. 759. 53.4608 .0213 .85 .12 16.47 0 .0000 1
2 5 .000 315.36 2.80 19.053 830. 830. 53.2705 .0213 .92 .10 15.36 0 .0000 1

FRMX= .127 IFRMX= 3      FRMN= .103 IFRMN= 5
RIVER= 3      QU(1)= 53.000      YU(1)= 312.54      QU(N)= 53.644      YU(N)= 311.8
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
3 1 22.400 312.54 1.48 35.886 1638. 1638. 53.0000 .0184 2.51 .06 22.54 0 .0000 1
3 2 16.800 312.35 1.63 32.474 1453. 1453. 53.0368 .0184 2.49 .06 22.35 0 .0000 1
3 3 11.200 312.17 1.62 32.790 1392. 1392. 53.0819 .0184 2.49 .06 22.17 0 .0000 1
3 4 5.600 312.00 1.53 34.785 1430. 1430. 53.1536 .0183 2.49 .05 22.00 0 .3820 1
3 5 .000 311.80 1.80 29.770 1304. 1304. 53.6442 .0182 2.47 .07 21.80 0 .0000 1

FRMX= .066 IFRMX= 5      FRMN= .055 IFRMN= 4
RIVER= 4      QU(1)= 209.000      YU(1)= 354.86      QU(N)= 208.413      YU(N)= 303.7
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
4 1 109.900 354.86 4.63 45.134 2108. 2108. 209.0000 .0336 .21 .18 24.86 0 .0000 1
4 2 106.800 352.57 3.73 55.929 2617. 2617. 208.8759 .0336 .68 .14 22.57 0 .0000 1
4 3 94.100 345.92 3.59 58.038 2154. 2154. 208.3894 .0262 .90 .12 20.92 0 .0000 1
4 4 88.000 343.70 4.17 49.982 2350. 2350. 208.1935 .0262 1.32 .16 18.70 0 .0000 1
.
.
.
4 9 30.000 312.55 4.25 50.104 2563. 2563. 212.9984 .0317 .80 .17 12.55 0 .0000 1
4 10 15.000 307.51 2.39 88.544 3515. 3888. 211.3783 .0317 1.41 .08 27.51 0 .0000 1
4 11 .000 303.73 3.25 64.068 2557. 2605. 208.4129 .0317 3.29 .11 23.73 0 .0000 1

FRMX= .187 IFRMX= 6      FRMN= .084 IFRMN= 10

TOTAL INFLOW (1000 CF)      TOTAL OUTFLOW (1000 CF)      TOTAL VOLUME      CONTINUITY ERROR
RIVER      TRIBUTARIES      RIVER      TRIBUTARIES      CHANGE(1000 CUFT)      (PERCENT)
81590020.00      85050680.00      158817100.00      81978020.00      4884262.00      1.76

TOTAL VOLUME/ACTIVE VOLUME CHANGE (%) OF RIVER 1 =      94.65      242.67

TRIBUTARY ITERATIONS = 5
TOTAL ITERATIONS FOR EACH OF 4 RIVERS.
37      8      17      11

TOTAL TIME= 48.00      TOTAL NO. OF TIME STEPS: KTIME= 3      NUMTIM= 3
    
```

Definition of Variables in Outflow Summary

- KTIME - Total number of time steps used in the computations
- NUMTIM - Total number of time steps stored for use in FLDGRF model

Figure 21. Counters after interpolation information

NEW INPUT CROSS SECTION NO. AFTER INTERPOLATION

RIVER NO. 1
 NN= 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20

RIVER NO. 1
 NGS= 1 3 5 7 12 14 16 20
 LQ1= 9
 LQN= 10

L= 1 KRTYP= 0 KRT1= 1 KRTN= 3
 L= 2 KRTYP= 0 KRT1= 3 KRTN= 5
 L= 3 KRTYP= 0 KRT1= 5 KRTN= 7
 L= 4 KRTYP= 0 KRT1= 7 KRTN= 12
 L= 5 KRTYP= 0 KRT1= 12 KRTN= 14
 L= 6 KRTYP= 0 KRT1= 14 KRTN= 16
 L= 7 KRTYP= 0 KRT1= 16 KRTN= 20

(SLOP(I,J),I=1,N) FOR RIVER NO. 1
 .000001 .000168 .000001 .000042 .000001 .000046 .000001 .000001
 .000001 .000001 .000022 .000001 .000021 .000001 .000031 .000001
 .000001 .000001 .000001 .000001

QDI(I, 1)
 150600. 0. 0. 0. 0. 0. 0. 0.
 0. 0. 0. 0. 0. 0. 0. 0.
 0. 0. 0. 0.

YDI(I, 1)
 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 -.26

Definition of Variables in Counters after Interpolation Information

- RIVER NO. - River number
- NGS - Gage locations
- LQ1 - Beginning location for lateral flow
- LQN - Ending location for lateral flow
- L - Counter for different routing techniques (simulation mode) or for calibration reaches (calibration mode)
- KRTYP - Routing type
- KRT1 - Beginning location of the routing/calibration reach
- KRTN - Ending location of the routing/calibration reach
- SLOP - Slope of channel
- I - Cross section counter
- J - River number
- QDI - Initial discharges (CFS or CMS)
- YDI - Initial water surface elevation (FT or M)

Figure 22. Levee information after interpolation

L	NJFM	NIFM	NJTO	NITO	X	HWL	TFLV	WCLV	BLVMX	HFLV	HLVMN	SLV	HPLV	DPLV
1	1	3	1	0	10.00	107.00	3.00	2.50	125.00	1000.00	105.00	.00010	.00	.00
2	1	4	1	0	11.25	106.34	3.00	2.50	125.00	999.34	104.34	.00010	.00	.00
3	1	5	1	0	12.50	105.68	3.00	2.50	125.00	998.68	103.68	.00010	.00	.00
4	1	6	1	0	13.75	105.02	3.00	2.50	125.00	998.02	103.02	.00010	.00	.00
5	1	3	2	2	10.00	107.00	3.00	2.50	125.00	1000.00	105.00	.00010	.00	.00
.														
22	1	11	3	0	25.00	91.00	3.00	2.50	1000.00	91.00	89.00	.00010	.00	.00
23	1	12	3	0	26.00	90.47	3.00	2.50	1000.00	90.47	88.47	.00010	.00	.00
24	1	13	3	0	27.00	89.94	3.00	2.50	1000.00	89.94	87.94	.00010	.00	.00
25	1	14	3	0	28.00	89.42	3.00	2.50	1000.00	89.42	87.42	.00010	.00	.00
26	1	15	3	0	29.00	88.89	3.00	2.50	1000.00	88.89	86.89	.00010	.00	.00

Definition of Variables in Levee Information after Interpolation

- L - Levee counter
- NJFM - Number of river passing levee overtopping/failure flow
- NIFM - Number of reach along the river with levee passing flow
- NJTO - Number of river receiving flow from levee overtopping/failure
- X - Cross section location (MI or KM)
- HWL - Elevation (FT or M) of top of levee, ridge line, etc.
- TFLV - Time of levee failure (crevasse) WCLV - Weir-flow discharge coefficient (levee)
- BLVMX - Final width of levee crevasse
- HFLV - Elevation of water surface (FT or M) when levee starts to fail (FT or M)
- HLVMN - Final elevation of bottom of levee crevasse (FT or M)
- SLV - Slope of the levee
- HPLV - Centerline elevation (FT or M) of flood drainage pipe (levee)
- DPLV - Diameter of flood drainage pipe

Figure 23. Mixed flow analysis output

```

TT = .00000 HRS      DTH = .00415 HRS      ITMX= 0
RIVER= 1      QU(1)= 1.000      YU(1)= 1549.00      QU(N)= 1.000      YU(N)= 720.18
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCF3) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCF3) MRV
1 1 .000 1549.00 .12 8.488 243. 243. 1.0000 .0350 .00 .00 44.00 10 .0000 0
1 2 .010 1508.55 8.27 .121 62. 62. 1.0000 .0350 .00 1.04 3.55 0 .0000 0
1 3 .064 1502.86 9.36 .107 62. 62. 1.0000 .0350 .00 1.26 2.23 0 .0000 0
1 4 .117 1498.24 7.29 .137 81. 81. 1.0000 .0350 .00 .99 1.97 0 .0000 0
1 5 .171 1493.41 7.36 .136 98. 98. 1.0000 .0350 .00 1.10 1.51 0 .0000 0
1 6 .225 1489.03 5.95 .168 121. 121. 1.0000 .0350 .00 .89 1.48 0 .0000 0
1 7 .278 1484.32 6.52 .153 140. 140. 1.0000 .0350 .00 1.10 1.13 0 .0000 0
1 8 .332 1480.01 5.28 .189 163. 163. 1.0000 .0350 .00 .86 1.19 0 .0000 0
1 9 .385 1475.41 5.77 .173 185. 185. 1.0000 .0350 .00 1.05 .96 0 .0000 0
FRMX= 1.256  IFRMX= 3      FRMN= .004  IFRMN= 1

RESERVOIR OUTFLOW INFORMATION
J I TT QU(1) USH(MSL) YB(MSL) DSH(MSL) SUB BB QU(1) QBRECH QOVTOP QOTHR
1 1 .000 1.000 1549.00 1549.00 1508.55 1.00 .00 1.000 .000 .000 1.000

INITIAL CONDITIONS IMPROVED BY SOLVING UNSTEADY FLOW EQUATIONS WITH BOUNDARIES HELD CONSTANT
IFR= 0 2 1 2 1 0 1 0 2 0 2 0 2 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 0 2 0 2 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
ICTR= 0      0      0      0
IFR= 0 2 1 2 1 0 2 0 2 0 2 0 2 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 0 2 0 2 0 2 0 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
IFR= 0 2 1 2 1 1 2 2 2 2 2 2 2 2 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2 2 2 2 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
IFR= 0 2 1 2 1 1 2 2 2 2 2 2 2 2 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2 2 2 2 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
IFR= 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

L= 1      KSP= 0      KS1= 1      KSN= 2
L= 2      KSP= 1      KS1= 2      KSN= 17
L= 3      KSP= 0      KS1= 18     KSN= 36
L= 4      KSP= 1      KS1= 36     KSN= 42
L= 5      KSP= 0      KS1= 43     KSN=112
RIVER= 1      DYN/CUN REACH= 1      SUP/SUB FLOW REACH= 1      KIT = 2
MAX DQ AND MAX DY ARE FINAL MAXIMUM ERROR IN NEWTON RAPHSOIN ITERATION METHOD WHILE SOLVING ST. VENANT EQUATION
MAX DQ= .0 AT I= 0      MAX DY= .000 AT I= 0  ITER= 0      CFACT= 1.000
K= 1      I= 2      YNO= 1508.55      YNN= 1508.55      F= -1.3
.
.
MAX DQ AND MAX DY ARE FINAL MAXIMUM ERROR IN NEWTON RAPHSOIN ITERATION METHOD WHILE SOLVING ST. VENANT EQUATION
MAX DQ= .0 AT I= 0      MAX DY= .000 AT I= 0  ITER= 0      CFACT= 1.000
.61343E-02 .00000E+00 -.44164E-06 .61339E-02 .78322E-03 (TERM1,TERM2,TERM3,SF,SMIN)
RIVER= 1      DYN/CUN REACH= 1      SUP/SUB FLOW REACH= 5      KIT = 2
MAX DQ AND MAX DY ARE FINAL MAXIMUM ERROR IN NEWTON RAPHSOIN ITERATION METHOD WHILE SOLVING ST. VENANT EQUATION
MAX DQ= .0 AT I= 0      MAX DY= .000 AT I= 0  ITER= 0      CFACT= 1.000

```

Definition of Variables in Analysis Output

- TERM1 - First term in momentum equation
- TERM2 - Second term in momentum equation
- TERM3 - Third term in momentum equation
- SF - Friction slope
- SMIN - Minimum allowable friction slope
- MAX DQ - Maximum flow error using Newton Raphson technique
- MAX D - Maximum elevation error using Newton Raphson technique
- ITER - Iteration counter for Newton Raphson technique
- CFACT - Multiplier used to update the next guess for flow/water elevation in the Newton Raphson technique
- RIVER - River number
- DYN/CUN REACH - Routing type
- SUP/SUB FLOW REACH - Routing type
- REACH - Reach number contained in the routing type
- KIT - Iteration counter
- K - Iteration counter
- I - Cross section number
- YNO - Old guess for the normal water surface elevation (FT or M)
- YNN - New guess for the normal water surface elevation (FT or M)
- IFR - Flow regime
- 0 - Subcritical flow
- 1 - Supercritical flow
- 2 - Critical flow

Figure 24. Tributary analysis information

```

INITIAL CONDITIONS IMPROVED BY SOLVING UNSTEADY FLOW EQUATIONS WITH BOUNDARIES HELD CONSTANT
RIVER= 1 DYN/CUN REACH= 1 SUP/SUB FLOW REACH= 1 KIT = 2
MAX DQ AND MAX DY ARE FINAL MAXIMUM ERROR IN NEWTON RAPHSON ITERATION METHOD WHILE SOLVING ST. VENANT EQUATION
MAX DQ=***** AT I= 24 MAX DY= 4.492 AT I= 23 ITER= 0 CFACT= 1.000
MAX DQ=***** AT I= 24 MAX DY= .906 AT I= 17 ITER= 1 CFACT= 1.000
MAX DQ= .0 AT I= 24 MAX DY= .147 AT I= 13 ITER= 2 CFACT= 1.000
MAX DQ= .0 AT I= 24 MAX DY= .000 AT I= 13 ITER= 3 CFACT= 1.000
RIVER= 2 DYN/CUN REACH= 1 SUP/SUB FLOW REACH= 1 KIT = 2
MAX DQ= .0 AT I= 0 MAX DY= 1.250 AT I= 1 ITER= 0 CFACT= 1.000
MAX DQ= .0 AT I= 0 MAX DY= .000 AT I= 1 ITER= 1 CFACT= 1.000
RIVER= 3 DYN/CUN REACH= 1 SUP/SUB FLOW REACH= 1 KIT = 2
MAX DQ= .0 AT I= 0 MAX DY= 2.031 AT I= 1 ITER= 0 CFACT= 1.000
MAX DQ= .0 AT I= 0 MAX DY= .000 AT I= 1 ITER= 1 CFACT= 1.000
RIVER= 4 DYN/CUN REACH= 1 SUP/SUB FLOW REACH= 1 KIT = 2
MAX DQ= .0 AT I= 0 MAX DY= 1.080 AT I= 4 ITER= 0 CFACT= 1.000
MAX DQ= .0 AT I= 0 MAX DY= .000 AT I= 4 ITER= 1 CFACT= 1.000
RELX ITR JRIVER ERQ QOLD QNEW NEWTON ITRMX
0 2 619. 54181. 54800. 1
0 3 6205. 47573. 53778. 1
0 4 1209. 206791. 208000. 1

TT = .00000 HRS DTH = 24.00000 HRS ITMX= 3 1 1 1
RIVER= 1 QU(1)= 288.000 YU(1)= 335.12 QU(N)= 446.638 YU(N)= 255.79
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
1 1 1076.500 325.12 3.42 84.139 3064. 3064. 288.0000 .0309 -.45 .12 35.12 0 .0000 0
1 2 1067.300 332.66 2.88 99.700 4442. 4442. 287.5988 .0308 .51 .11 32.66 0 .3720 0
1 3 1058.000 328.77 3.64 80.934 3787. 3787. 294.7946 .0225 .07 .14 28.77 0 .0000 0
1 4 1049.900 327.33 3.40 86.226 3387. 3387. 293.1569 .0225 1.12 .12 32.33 0 .0000 0
1 5 1031.700 321.61 4.28 66.893 3694. 3694. 286.3499 .0195 1.01 .18 21.61 0 .0000 0
.
.
1 20 920.000 290.04 4.37 117.613 5011. 5011. 514.2587 .0248 2.62 .16 30.04 0 .0000 0
1 21 904.500 281.31 4.42 114.388 4689. 4689. 505.1895 .0248 -.24 .16 31.31 0 .0000 0
1 22 889.000 276.18 3.68 137.917 4698. 4698. 507.9590 .0262 -.50 .12 36.18 0 .0000 0
1 23 867.700 270.33 3.24 147.489 6912. 6912. 478.5753 .0262 4.59 .12 30.33 0 .0000 0
1 24 846.400 255.79 4.34 102.819 5328. 5328. 446.6384 .0262 .00 .17 15.79 0 .0000 0
FRMX= .270 IFRMX= 7 FRMN= .107 IFRMN= 2
RIVER= 2 QU(1)= 54.800 YU(1)= 320.39 QU(N)= 54.181 YU(N)= 313.67
J I X(MI) H(MSL) V(FPS) A(TSQFT) B(FT) BT(FT) Q(TCFS) MANN. N WAVHT FROUDE DEP(FT) KR QL(TCFS) MRV
2 1 30.600 320.39 3.22 17.045 720. 720. 54.8000 .0212 1.25 .12 20.39 0 .0000 1
2 2 23.000 318.97 3.08 17.565 811. 811. 54.1544 .0213 1.00 .12 18.97 0 .0000 1
2 3 15.300 317.10 3.37 15.948 803. 803. 53.7437 .0213 .31 .13 17.10 0 .0000 1
2 4 7.700 315.12 3.37 15.951 738. 738. 53.7915 .0213 -.50 .13 15.12 0 .0000 1
2 5 .000 313.67 3.07 17.669 802. 802. 54.1815 .0212 -.77 .12 13.67 0 .0000 1
FRMX= .133 IFRMX= 3 FRMN= .115 IFRMN= 5

```

Definition of Variables in Tributary Analysis Information

- RELX ITR - Iteration counter
- JRIVER - River number
- ERQ - Flow error
- QOLD - Flow at previous time step (CFS or CMS)
- QNEW - Flow at current time step (CFS or CMS)

Figure 25. Downwater analysis information

```

DOWNWATER  IN= 9      YNN= 506.91  DEP= 5.71
            ITD= 1    I= 10    YDWO= 498.12  YDWN= 498.48  F= 13278.2
            ITD= 2    I= 10    YDWO= 498.48  YDWN= 498.50  F= 806.1
            ITD= 3    I= 10    YDWO= 498.50  YDWN= 498.50  F= 3.2
            ITD= 1    I= 10    YIL= 506.91  QII= 6000.   YA= 494.98  F= 234737.9
            ITD= 2    I= 10    YIL= 506.91  QII= 6000.   YA= 496.57  F= 94487.3
            ITD= 3    I= 10    YIL= 506.91  QII= 6000.   YA= 497.36  F= 50248.1
            ITD= 4    I= 10    YIL= 506.91  QII= 6000.   YA= 497.76  F= 32323.5
            ITD= 5    I= 10    YIL= 506.91  QII= 6000.   YA= 497.96  F= 24246.5
            ITD= 6    I= 10    YIL= 506.91  QII= 6000.   YA= 498.06  F= 20412.3
            ITD= 7    I= 10    YIL= 506.91  QII= 6000.   YA= 498.11  F= 18544.5
            ITD= 8    I= 10    YIL= 506.91  QII= 6000.   YA= 498.13  F= 17623.3
            ITD= 9    I= 10    YIL= 506.91  QII= 6000.   YA= 498.14  F= 17165.2
            ITD= 10   I= 10    YIL= 506.91  QII= 6000.   YA= 498.15  F= 16937.4
            ITD= 11   I= 10    YIL= 506.91  QII= 6000.   YA= 498.15  F= 16823.2
            .
            .
            I= 12    QIR= 6000.   YIR= 481.35  DEP= 8.35    ITD= 12

BACKWATER  IN= 9      YNN= 506.91  DEP= 5.71
            ITB= 1    I= 8     YBWO= 517.84  YBWN= 515.79  F= -73417.240
            ITB= 2    I= 8     YBWO= 515.79  YBWN= 516.08  F= 13213.320
            ITB= 3    I= 8     YBWO= 516.08  YBWN= 516.08  F= 30.392
            I= 8     QIL= 6000.   YIL= 516.08  DEP= 5.48    ITB= 3
            ITB= 1    I= 7     YBWO= 526.82  YBWN= 525.19  F= -79298.310
            ITB= 2    I= 7     YBWO= 525.19  YBWN= 524.73  F= -14139.490
            ITB= 3    I= 7     YBWO= 524.73  YBWN= 524.82  F= 4193.991
            ITB= 4    I= 7     YBWO= 524.82  YBWN= 524.82  F= 6.298
            .
            .
            I= 3     QIL= 6000.   YIL= 589.18  DEP= 5.85    ITB= 3
            ITB= 1    I= 2     YBWO= 607.14  YBWN= 605.69  F= -207120.900
            ITB= 2    I= 2     YBWO= 605.69  YBWN= 605.67  F= -1737.361
            ITB= 3    I= 2     YBWO= 605.67  YBWN= 605.67  F= 1.942
            I= 2     QIL= 6000.   YIL= 605.67  DEP= 6.51    ITB= 3
            ITB= 1    I= 1     YBWO= 623.30  YBWN= 621.57  F= -249028.800
            ITB= 2    I= 1     YBWO= 621.57  YBWN= 621.52  F= -7117.417
            ITB= 3    I= 1     YBWO= 621.52  YBWN= 621.52  F= -10.743
            I= 1     QIL= 6000.   YIL= 621.52  DEP= 6.52    ITB= 3
    
```

Definition of Variables in Downwater Analysis Information

- IN - Beginning cross section number for downwater computations
- YNN - Water surface elevation at initial boundary
- DEP - Water depth
- ITD - Iteration counter
- I - Cross section number
- YDWO - Old guess for the water surface elevation (FT or M)
- YDWN - New guess for the water surface elevation (FT or M)
- F - Difference between the computed discharge and the actual discharge
- YIL - Initial water surface elevation (FT or M)
- QII - Initial discharge (CFS or CMS)
- YA - Average water surface elevation within the reach (FT or M)

Figure 26. Conveyance analysis information

```

GENERATING CONVEYANCE CURVE

QKT (K) =      0.      4.      23.      67.      144.      261.      425.      641.
QKT (K) =     915.     1253.     1659.     2139.     2698.     3340.     4070.     4892.
QKT (K) =    5811.     6830.     7955.     9189.     10535.     11999.     13584.     15294.
.
.
QKT (K) =   6281732.   6348753.   6279316.   6180960.   6090048.   6005922.   5927997.   5855756.
QKT (K) =   5788739.   5743179.   5729040.   5716623.   5705842.   5696618.   5688877.   5682550.
QKT (K) =   5677571.   5673881.   5671424.

QKT (K) =      0.      4.      23.      67.      144.      261.      425.      641.
QKT (K) =     915.     1253.     1659.     2139.     2698.     3340.     4070.     4892.
QKT (K) =    5811.     6830.     7955.     9189.     10535.     11999.     13584.     15294.
.
.
QKT (K) =   6281732.   6348753.   6308910.   6269067.   6229224.   6189381.   6149538.   6109695.
QKT (K) =   6069852.   6030009.   5990166.   5950323.   5910480.   5870637.   5830794.   5790951.
QKT (K) =   5751108.   5711265.   5671424.

BKT (K) =     1.060     1.124     1.076     1.067     1.064     1.063     1.062     1.061
BKT (K) =     1.061     1.061     1.061     1.061     1.060     1.060     1.060     1.060
BKT (K) =     1.060     1.060     1.060     1.060     1.060     1.060     1.060     1.060
.
.
BKT (K) =     1.140     1.140     1.140     1.140     1.140     1.140     1.140     1.140
BKT (K) =     1.140     1.139     1.139     1.139     1.139     1.139     1.139     1.138
BKT (K) =     1.138     1.138     1.138

J=  1      I=  1      L=  30      ERQK=  2.15      NKC (I, J) =  30

HKC (L, I, J) =   500.10   500.36   500.61   500.87   501.12   501.38   501.64   501.89
HKC (L, I, J) =   502.15   502.40   502.66   503.17   503.68   504.45   505.22   506.24
HKC (L, I, J) =   507.52   509.06   510.08   511.87   513.66   515.71   518.27   521.08
HKC (L, I, J) =   524.67   528.76   533.88   541.04   545.65   549.74

QKC (L, I, J) =      0.      4.      23.      67.      144.      261.      425.      641.
QKC (L, I, J) =     915.     1253.     1659.     2698.     4070.     6830.     10535.     17132.
QKC (L, I, J) =   28377.   46855.   62858.   106576.   166867.   261695.   427088.   679722.
QKC (L, I, J) =  1125954.  1832946.  3056293.  5309196.  6308910.  5671424.

BEV (L, I, J) =     1.060     1.124     1.076     1.067     1.064     1.063     1.062     1.061
BEV (L, I, J) =     1.061     1.061     1.061     1.060     1.060     1.060     1.060     1.060
BEV (L, I, J) =     1.060     1.060     1.060     1.102     1.147     1.177     1.190     1.188
BEV (L, I, J) =     1.177     1.164     1.149     1.137     1.140     1.138

SNM (K,  1,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNM (K,  2,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNM (K,  3,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
.
.
SNM (K, 88,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNM (K, 89,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00

ERQMX=  2.14  2.13  2.15  2.13  2.15  2.14  2.15  2.15  2.14  1.79*  1.79  1.79  1.79  1.79  1.79  1.78  1.79  1.78  1.78  1.79
ERQMX=  1.78  1.79  1.79  1.78  1.49  1.49  1.48  1.48  1.49  1.49*  1.49  1.49  1.49  1.49  1.24  1.24  1.24  1.24  1.24  1.48
ERQMX=  1.24  1.49  1.46  1.49  1.49  1.24  1.24  1.49  1.24  1.24*  1.24  1.03  1.24  1.24  1.24  1.24  1.24  1.24  1.03  1.03
ERQMX=   .86  1.03   .86   .86   .86   .86   .72   .72   .72   .72*   .72   .72   .72   .72   .72   .71   .72   .71  1.79   .86
ERQMX=   .86   .86   .86   .86   .86   .85   .86   .86   .86

SNC (K,  1,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNC (K,  2,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNC (K,  3,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
.
.
SNC (K, 87,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNC (K, 88,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00
SNC (K, 89,  1) =   1.00   1.00   1.00   1.00   1.00   1.00   1.00

```

Definition of Variables in Conveyance Analysis Information

- QKT(K) - Discharges in the initial conveyance curve (CFS or CMS)
- BKT(K) - Initial beta correction coefficient used in the momentum equation
- J - River number
- I - Cross section number
- L - Final number of points in the conveyance table
- ERQK - Flow difference between two points on the conveyance curve
- NKC(I,J) - Number of points in the conveyance curve for cross section I on

river J
HKC(L,I,J) - Elevations in the final conveyance table (FT or M)
QKC(L,I,J) - Discharges in the final conveyance curve (CFS or CMS)
BEV(L,I,J) - Final beta correction coefficient used in the momentum equation
SNM - Sinuosity coefficient used in the momentum equation
ERQMX - Error in maximum flow
SNC - Sinuosity coefficient used in the continuity equation

Figure 27. Cross section analysis information

```

** COMPUTE INITIAL FLOW, NORMAL AND INITIAL DEPTH FOR RIVER NO 1 **

(QDI(I,1),I=1,N)
288000. 288000. 296372. 296372. 296372. 296372. 296372. 351172.
351172. 351172. 404950. 404950. 404950. 404950. 404950. 404950.
405724. 613724. 613724. 613724. 613724. 613724. 613724. 613724.

(QDI(I,2),I=1,N)
54800. 54800. 54800. 54800. 54800.

(QDI(I,3),I=1,N)
53200. 53200. 53200. 53778.

(QDI(I,4),I=1,N)
200000. 200000. 200000. 200000. 200000. 208000. 208000. 208000.
208000. 208000. 208000.

INITIAL DISCHARGES:

(QDI FOR RIVER NO. 1
288000. 288000. 296372. 296372. 296372. 296372. 296372. 351172.
351172. 351172. 404950. 404950. 404950. 404950. 404950. 404950.
405724. 613724. 613724. 613724. 613724. 613724. 613724. 613724.

** COMPUTE NORMAL/CRITICAL DEPTH **
AS(1, 1 1)= 13820. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 2 1)= 11627. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 3 1)= 9750. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 4 1)= 5727. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 5 1)= 10243. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 6 1)= 15683. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 7 1)= 6085. > 0.0; SUB-CRITICAL FLOW ASSUMED!
I= 8 X= 1014.500 YN= 314.37 DEPN= 24.37 YC= 297.72 DEPC= 7.72 IFR= 0 ITN= 0 ITC= 14
AS(1, 9 1)= 16740. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 10 1)= 20344. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 11 1)= 5440. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 12 1)= 8234. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 13 1)= 12807. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 14 1)= 12983. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 15 1)= 15838. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 16 1)= 7059. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 17 1)= 22460. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 18 1)= 23569. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 19 1)= 14143. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 20 1)= 20793. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 21 1)= 18379. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 22 1)= 22182. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 23 1)= 21317. > 0.0; SUB-CRITICAL FLOW ASSUMED!
AS(1, 24 1)= 28453. > 0.0; SUB-CRITICAL FLOW ASSUMED!

```

Definition of Variables in Cross Section Analysis Information

- AS - Area of cross section (FT² or M²)
- I - Cross section number
- X - Cross section location (MI or KM)
- YN - Normal flow water surface elevation (FT or M) for initial flow at t=0
- DEPN - Normal flow depth (FT or M) for initial flow
- YC - Critical flow water surface elevation (FT or M) for initial flow at t=0
- DEPC - Critical flow depth (FT or M)
- IFR - Froude number indicator: 0 indicates Fr<1, 1 indicates Fr>=1
- ITN - Number of iterations to obtain YN via bi-section solution method
- ITC - Number of iterations to obtain YC via bi-section solution method

Figure 28. Cross section calibration analysis information

```

DOUBLE MAX ITERATION FOR CALIBRATION, ITMAX= 20

RIVER NO. 1 REACH NO. 1 JACKSON MI 216 STA NO. 1 RIVER MILE= 216.000
RIVER NO. 1 MANNING N REACH NO. 1
FKC= 61.8575 FMC= .2500 FKF= 57.3247 FMF= .4000 FKO= .0000 FMO= .0000
X(I,J) 216.00 210.00 205.00 200.00 199.99 194.99 190.00 185.00
X(I,J) 184.00 183.00 182.00 181.00 180.00 179.99 179.00 178.00
IFXC= 0 0 0 0 0 0 0 0
IFXC= 0 0 0 0 0 1 1 1
HS= 906.30 908.30 916.30 920.30 936.30 1036.30
BS= .00 73.56 110.00 209.81 300.00 300.00
AS= 0. 74. 808. 1447. 5526. 35526.
TT = .00000 DTH = 6.00000 HRS ITMX= 0
RIVER= 1 MANNING REACH= 1 QU(1)= .094 YU(1)= 908.54 QU(N)= .300 YU(N)= 854.68
J I X Y V A B BT Q CMM
1 1 216.00 908.54 1.03 92. 75. 75. .09 .0400
1 2 210.00 903.39 .52 180. 80. 80. .09 .0400
1 3 205.00 896.26 2.25 42. 55. 55. .09 .0400
1 4 200.00 894.06 .34 277. 85. 85. .09 .0400
.
1 11 182.00 883.23 .25 1183. 176. 176. .30 .0400
1 12 181.00 883.21 .22 1378. 201. 201. .30 .0400
1 13 180.00 883.20 .19 1596. 214. 214. .30 .0400
1 14 179.99 858.97 1.13 266. 110. 110. .30 .0400
1 15 179.00 856.11 1.74 172. 110. 110. .30 .0400
1 16 178.00 854.68 .73 409. 160. 160. .30 .0400

```

Definition of Variables in Cross Section Calibration Analysis Information

- FKC - Scaling parameter of in-bank channel portion of cross section
- FMC - Shape factor for in-bank channel portion of cross section
- FKF - Scaling parameter of flood plain portion of cross section
- FKO - Scaling parameter of inactive portion of cross section
- FMO - Shape factor for inactive portion of cross section
- X - Cross section location (MI or KM)
- I - Cross section number
- J - River number
- IFXC - Parameter indicating if cross sections have special properties
- HS - Height of section (FT or M)
- BS - Topwidth of section (FT or M)
- AS - Area of cross section (FT² or M²)
- TT - Time at which output is given (HR)
- DTH - Computational time step
- ITMX - Number of iterations in Newton Raphson solution of Saint-Venant equations
- RIVER - River number
- QU(1) - Discharge (CFS or CMS) at upstream boundary
- YU(1) - Water surface elevation (FT or M) at upstream boundary
- QU(N) - Discharge (CFS or CMS) at downstream boundary
- YU(N) - Water surface elevation (FT or M) at downstream boundary
- Y - Water surface elevation (FT or M) in cross section I
- V - Velocity (fps or mps) in cross section I
- A - Active area in cross section I (FT² or M²)
- B - Active topwidth in cross section I (FT or M)
- BT - Total topwidth in cross section I (FT or M)
- Q - Discharge (CFS or CMS)

Figure 29. Hydrograph plot

RIVER = 4 STATION = 1 GLMSW STG FLOOD ELEV = 54.00 FEET															
MAX ELEV = 53.15 FEET AT TIME = 114.000 HOUR															
MAX FLOW = 2290.79 CFS AT TIME = 108.000 HOUR															
*----COMPUTED +----OBSERVED ELEV (FEET)															
TIME	44.	46.	48.	50.	52.	54.	56.	58.	60.	62.	64.	Q-FCST	H-FCST	H-OBS	TIME
3/29/11+	*	!03	44.91	44.10	6.000
3/29/17+	*	!03	44.91	44.12	12.000
3/29/23.	+	*	.	.	.	!09	46.14	45.65	18.000
3/30/ 5.	.	+	.	.	.	!16	47.05	47.05	24.000
3/30/11.	.	.	*+	.	.	!25	47.73	48.09	30.000
3/30/17.	.	.	.	* +	.	!35	48.32	48.90	36.000
3/30/23.	.	.	.	* +	.	!48	48.88	49.61	42.000
3/31/ 5.	.	.	.	* +	.	!63	49.40	50.28	48.000
3/31/11.	.	.	.	* +	.	!79	49.85	50.88	54.000
3/31/17.	.	.	.	* +	.	!91	50.17	51.18	60.000
3/31/23.	.	.	.	* +	.	!	1.03	50.42	51.40	66.000
4/ 1/ 5.	.	.	.	* +	.	!	1.20	50.80	51.67	72.000
4/ 1/11.	.	.	.	* +	.	!	1.42	51.90	51.97	78.000
4/ 1/17.	.	.	.	* +	.	!	1.67	52.44	52.22	84.000
4/ 1/23.	.	.	.	* +	.	!	1.96	52.76	52.46	90.000
4/ 2/ 5.	.	.	.	* +	.	!	2.28	53.07	52.70	96.000
4/ 2/11.	.	.	.	* +	.	!	2.29	53.14	-999.00	102.000
4/ 2/17.	* !	2.29	53.14		108.000
4/ 2/23.	* !	2.29	53.15		114.000
4/ 3/ 5.	* !	2.28	53.14		120.000